# Structure II Recitation 3/8 

Three Moment Theorem

## Before we start ...

## Today's Tasks:

Homework Example (Three Moment Theorem) (12 Questions)
Lab Session (Continuous Beam)

## Reminder:

Preliminary Report Resubmission: 3/11, Monday (Email Submission)
Tower Testing: 3/20, Wednesday

Use the Three Moment Theorem to determine all reactions and support moments for the given continuous beam.

DATASET: 1 -2- $-3-$

| Span A | 16 FT |
| :--- | :---: |
| Span B | 24 FT |
| Span C | 15 FT |
| Uniform load on span A, w1 | 7 KLF |
| Uniform load on span C, w2 | 3 KLF |
| Point load on span b, P | 68 K |
| Distance to point load P from R2, D | 18 FT |

18 FT


## Three-Moment Theorem

- Any number of spans
- Symmetric or non-symmetric


## Procedure:

1. Draw a free body diagram of the first two spans.
2. Label the spans L1 and L2 and the supports (or free end) A, B and C as show.
3. Use the Three-Moment equation to solve for each unknown moment, either as a value or as an equation.


$$
M_{A} L_{1}+2 M_{B}\left(L_{1}+L_{2}\right)+M_{C} L_{2}=6\left[E I \Theta_{1}+E I \Theta_{2}\right]
$$

## Three-Moment Theorem

Procedure (continued):
4. Move one span further and repeat the procedure.
5. In a 3 span beam, the mid-moment from step 3 above (B), can now be solved using the two equations from step 4 and 3 together, by writing 2 equations with 2 unknowns.
6. Repeat as needed, always moving one span to the right and writing a new set of moment equations.
7. Solve 2 simultaneous equations for 3 spans, or 3 equations for more than 3 spans, to get the interior moments.
8. Once all interior moments are known, solve for reactions using free body diagrams of individual spans.
9. Draw shear and moment diagrams as usual. This will also serve as a check for the moment values.

$$
M_{A} L_{1}+2 M_{B}\left(L_{1}+L_{2}\right)+M_{C} L_{2}=6\left[E I \Theta_{1}+E I \Theta_{2}\right]
$$

## Continuous Beam

Q1: Moment at support R1 (M1)
Since its end support, $\underline{\mathbf{M 1}=\mathbf{0}}$

## Q2: EIO on Left Side of R2

Based on the given data:
$M_{A}=0($ From Q1)
$\mathrm{L} 1=\operatorname{Span} \mathrm{A}=16 \mathrm{ft}$
$\mathrm{L} 2=\operatorname{Span} \mathrm{B}=24 \mathrm{ft}$
Use the slope diagram to find the EI $\theta$ (Course Slides P.8, 3/4)
$\operatorname{EI} \theta(\mathrm{Left})=\mathrm{W} \times \mathrm{L}^{2} / 24=(7 \times 16) \times 16^{2} / 24=\underline{1194.667}$

Span A
16 FT
24 FT
15 FT
7 KLF
3 KLF
68 K
18 FT

## Q3: EI日 on Right Side of R2

$\operatorname{EI} \theta($ Right $)=5 \times \mathrm{P} \times \mathrm{L}^{2} / 128=5 \times 68 \times 24^{2} / 128=\underline{\mathbf{1 5 3 0}}$
 $M_{A} L_{1}+2 M_{B}\left(L_{1}+L_{2}\right)+M_{C} L_{2}=6\left[E I \Theta_{1}+E I \Theta_{2}\right]$

## Continuous Beam

Q4: Moment at support R4 (M4)
Since its end support, $\underline{\mathbf{M 4}=\mathbf{0}}$

## Q5: EIA on Left Side of R3

Based on the given data:
Mc=0(From Q4)
$\mathrm{L} 1=\operatorname{Span} \mathrm{B}=24 \mathrm{ft}$
$\mathrm{L} 2=\operatorname{Span} \mathrm{C}=15 \mathrm{ft}$
Use the slope diagram to find the EIO:
$\operatorname{EI} \theta($ Left $)=7 \times \mathrm{P} \times \mathrm{L}^{2} / 128=7 \times 68 \times 24^{2} / 128=\underline{\mathbf{2 1 4 2}}$

Span A


- Exterior end moments = 0
- Interior support moments are usually negative
- Mid-span moments are usually positive
$-E n d+M i d=0.125 w L^{2}$

Span B
Span C
Uniform load on span A, w1
Uniform load on span C, w2

## Point load on span b, P

Distance to point load P from R2, D

16 FT
24 FT
15 FT
7 KLF
3 KLF
68 K
18 FT

Q6: EI日 on Right Side of R3
$\operatorname{EI} \theta($ Right $)=W \times$ L $^{2} / 24=(3 \times 15) \times 15^{2} / 24=\underline{421.875}$

$M_{A} L_{1}+2 M_{B}\left(L_{1}+L_{2}\right)+M_{C} L_{2}=6\left[E I \Theta_{1}+E I \Theta_{2}\right]$

## $M_{A} L_{1}+2 M_{B}\left(L_{1}+L_{2}\right)+M_{C} L_{2}=6\left[E I \Theta_{1}+E I \Theta_{2}\right]$

## Q7 \& Q8 ( $\mathbf{M}_{2} \& \mathbf{M}_{3}$ )

Plug in the values into the Three Moment Theorem:

## First Formula:

$\mathrm{M}_{\mathrm{A}}=0, \mathrm{~L} 1=16 \mathrm{ft}, \mathrm{L} 2=24 \mathrm{ft}, \mathrm{EI} \theta_{1}=1194.667, \mathrm{EI} \theta_{2}=1530, \mathrm{Mb}=\mathrm{M}_{2}, \mathrm{Mc}=\mathrm{M}_{3}$
$0 \times 16+2 \times \mathrm{M}_{2} \times(16+24)+\mathrm{M}_{3} \times 24=6 \times(1194.667+1530)$
$\mathbf{8 0} \times \mathrm{M}_{2}+\mathbf{2 4} \times \mathrm{M}_{\mathbf{3}}=\mathbf{1 6 3 4 8 . 0 0 2}$

Second Formula:
$\mathrm{Mc}_{\mathrm{c}}=0, \mathrm{~L} 1=24 \mathrm{ft}, \mathrm{L} 2=15 \mathrm{ft}, \mathrm{EI} \theta_{1}=2142, \mathrm{EI} \theta_{2}=421.875, \mathrm{M}_{\mathbf{A}}=\mathrm{M}_{2}, \mathrm{Mb}=\mathrm{M}_{3}$
$\mathrm{M}_{2} \times 24+2 \times \mathrm{M}_{3} \times(24+15)+0 \times 15=6 \times(2142+421.875)$
$\mathbf{2 4} \times \mathrm{M}_{2}+\mathbf{7 8} \times \mathrm{M}_{\mathbf{3}}=\mathbf{1 5 3 8 3 . 2 5}$

Both tension on top:
$\underline{\mathrm{M} 2=-148.00635 \mathrm{k}-\mathrm{ft}, \mathrm{M} 3=-159.948 \mathrm{k}-\mathrm{ft}}$

Sign convention

## Q9 Support Reaction R1

Draw Free Body Diagram

| Span A | 16 FT |
| :--- | :---: |
| Span B | 24 FT |
| Span C | 15 FT |
| Uniform load on span A, w1 | 7 KLF |
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$\Sigma \mathrm{M}$ at $\mathrm{R} 2=0$ :
$\underline{\mathrm{R} 1 \times \operatorname{Span} \mathrm{A}+\mathrm{M}_{2}-(\mathrm{w} 1 \times \operatorname{Span} \mathrm{A}) \times(\operatorname{Span} A / 2)=0}$
$\mathrm{R} 1 \times 16+148.00635-(7 \times 16) \times(16 / 2)=0$
$\mathrm{R} 1=(112 \times 8-148.00635) / 16=\underline{46.75 k}$

$\mathrm{R} 1-(\mathrm{w} 1 \times \mathrm{L})+\mathrm{V} 2=0$
$\mathrm{V} 2=(7 \times 16)-46.75=65.25$

## Q10: Support Reaction R2

Draw Free Body Diagram
$\Sigma \mathrm{M}$ at $\mathrm{R} 3=0$ :
R2 x Span B + M3-P x (Span B - D) - M2 - V2 x (Span B $)=0$
$\mathrm{R} 2 \times 24+159.948-68 \times(24-18)-148.00635-65.25 \times 24=0$
$\mathrm{R} 2=(408+148.00635+1566-159.948) / 24=\underline{\mathbf{8 1 . 7 5} \mathbf{k}}$


Q12: Support Reaction R4 Draw Free Body Diagram

Span A
24 FT

Distance to point load P from R2, D

$\Sigma \mathrm{M}$ at R3 $=0$ :
$($ w $2 \times \operatorname{Span} C) \times($ Span C / 2) $-M 3-R 4 \times \operatorname{Span} C=0$
$(3 \times 15) \times(15 / 2)-159.948-\mathrm{R} 4 \times 15=0$
$\mathrm{R} 4=(337.5-159.948) / 15=\underline{\mathbf{1 1 . 8 3 7} \mathbf{k}}$

## Q11: Support Reaction R3

Look at the whole beam
$\Sigma \mathrm{Fy}=0:$
$(\mathrm{R} 1+\mathrm{R} 2+\mathrm{R} 3+\mathrm{R} 4)=(\mathrm{w} 1 \times \operatorname{Span} \mathrm{A})+\mathrm{P}+(\mathrm{w} 2 \times \operatorname{Span} \mathrm{C})$ $(46.75+81.75+\mathrm{R} 3+11.837)=7 \times 16+68+3 \times 15$
$R 3=\underline{84.663 \mathrm{k}}$

[visible confusion]

## Continuous Beams

## Description

This project uses observation to understand the behavior of beams continuous over multiple supports.

## Goals

To observe the behavior of continuous beams under different loadings
To estimate locations of contraflexure and effective lengths
To determine areas of positive and negative moment based on curvature

## Procedure

1. Using the 24 inch stick, position the supports and loads (with your finger) as shown in the diagrams below. Hold the beam down on the reactions if it lifts up.
2. For each case observe and draw the elastic curve.
3. Label + and - curvature (moment) and points of contraflexure.
4. Estimate the effective lengths, $\ell$, across the beam. (between points of $M=0$ )

## Lab Session:

## Step 1: Draw the elastic curve <br> Step 2: Label curvature with + or - <br> Step 3: Label the points of contraflexure <br> Step 4: Estimate the effective lengths (Total L is 24 inch)



## Sign convention



