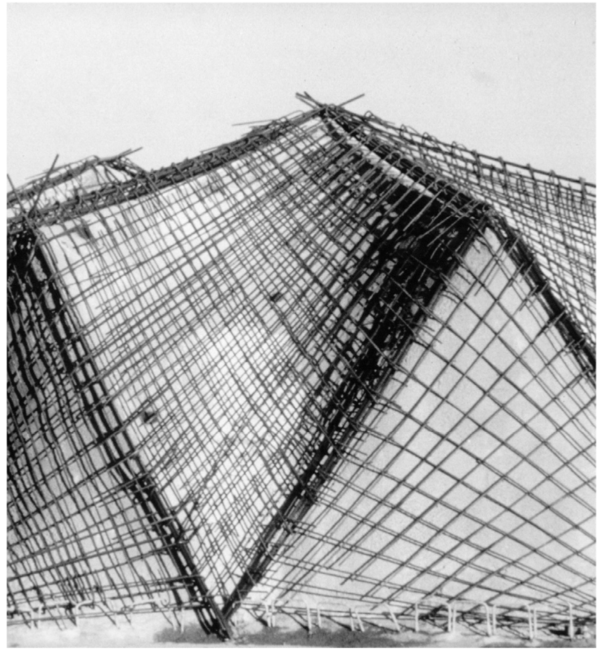
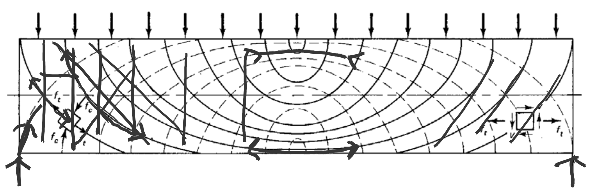
Architecture 324
Structures II

Reinforced Concrete Beams Ultimate Strength Design
(ACI 318-19) – PART I

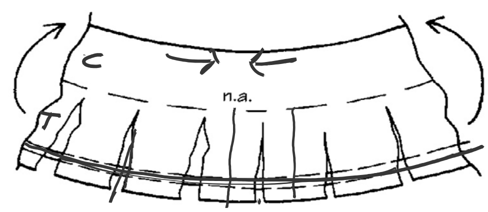


- Flexure in Concrete
- Ultimate Strength Design (LRFD)
- Failure Modes
- Flexure Equations
- Rectangular Beam Analysis

Flexure



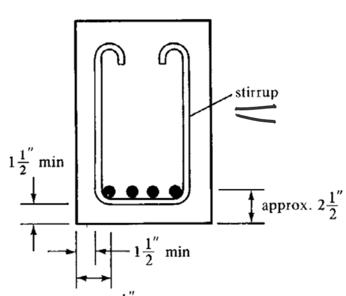
The stress trajectories in this simple beam, show principal tension as solid lines.

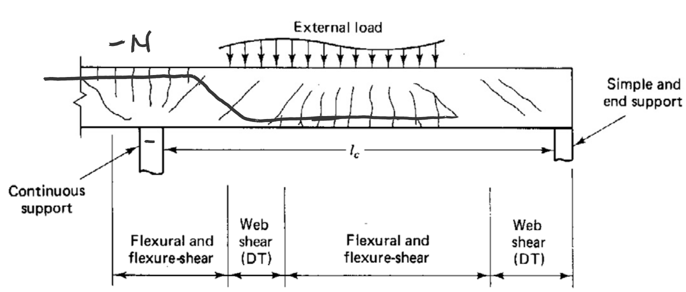


Reinforcement must be placed to resist these tensile forces

In beams continuous over supports, the stress reverses (negative moment).
In such areas, tensile steel is on top.

Shear reinforcement is provided by vertical or sloping stirrups.





Ultimate Strength – (LRFD)

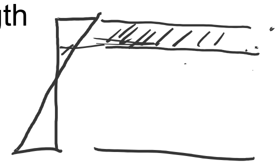
Nominal Strength ≥ Design Strength
(strength of member ≥ required by loads)

LRFD uses 2 safety factors: γ and ϕ
ϕ nominal strength ≥ γ required strength

γ increases the required strength of the member and is placed on the loads

ϕ reduces the member strength capacity and is placed on the calculated force

Loads increased:
γ Factors: DL=1.2 LL=1.6
U is the required strength
U=1.2DL+1.6LL
(factors from ASCE 7)



Strength reduced:
ϕ Factors: e.g. flexure = 0.9 in tension-controlled beams

Table 21.2.1—Strength reduction factors ϕ

	Action or structural element	ϕ	Exceptions
(a)	Moment, axial force, or combined moment and axial force	0.65 to 0.90 in accordance with 21.2.2	Near ends of pretensioned members where strands are not fully developed, ϕ shall be in accordance with 21.2.3.
(b)	Shear	0.75	Additional requirements are given in 21.2.4 for structures designed to resist earthquake effects.
(c)	Torsion	0.75	—
(d)	Bearing	0.65	—
(e)	Post-tensioned anchorage zones	0.85	—
(f)	Brackets and corbels	0.75	—
(g)	Struts, ties, nodal zones, and bearing areas designed in accordance with strut-and-tie method in Chapter 23	0.75	—
(h)	Components of connections of precast members controlled by yielding of steel elements in tension	0.90	—
(i)	Plain concrete elements	0.60	—
(j)	Anchors in concrete elements	0.45 to 0.75 in accordance with Chapter 17	—

Ultimate Strength – (ACI 318-14)

Reduced Nominal Strength ≥ Factored Load Effects

$$\Phi Sn \geq U$$

γ Factored Loads (see ACSE 7)

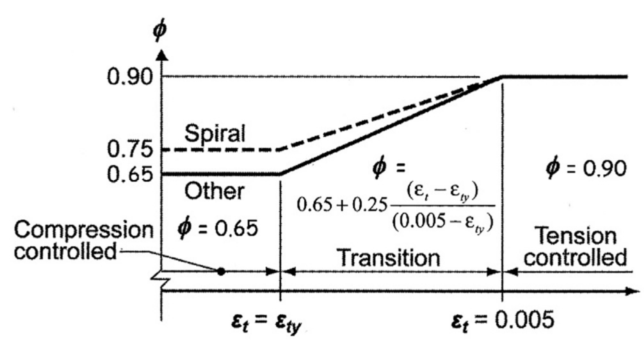
1) 1.4D
2) 1.2D + 1.6L + 0.5(Lr or S or R)
3) 1.2D + 1.6(Lr or S or R) + (1.0L or 0.5W)
4) 1.2D + 1.0W + 1.0L + 0.5(Lr or S or R)
5) 1.2D + 1.0E + 1.0L + 0.2S
6) 0.9D + 1.0W
7) 0.9D + 1.0E

D = service dead loads
L = service live load
Lr = service roof live load
S = snow loads
W = wind loads
R = rainwater loads
E = earthquake loads

Strength Reduction Factors, Φ

Mn	Flexural (ε > 0.005)	0.90
Vn	Shear	0.75
Pn	Compression (spiral)	0.75
Pn	Compression (other)	0.65
Bn	Bearing	0.65
Tn	Torsion	0.75
Nn	Tension	0.90
Combined stress		0.65 to 0.90

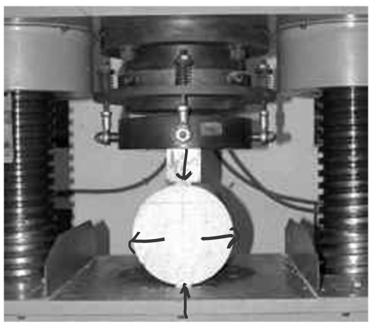
ACI 318 21.2.2

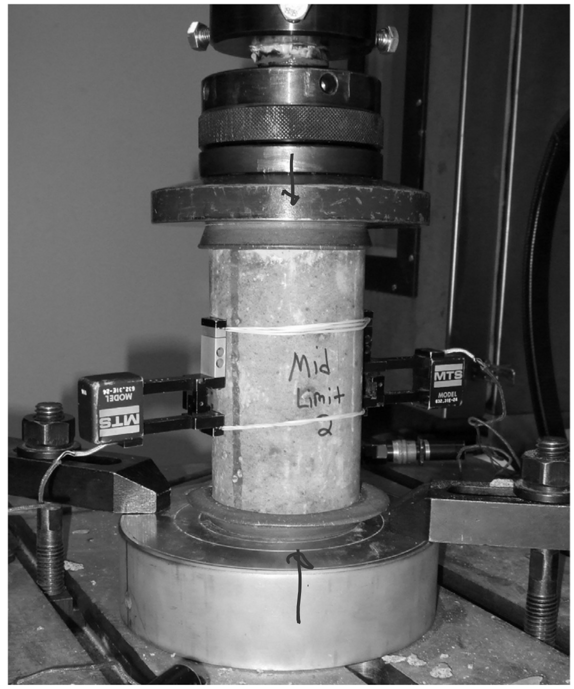


Strength Measurement

- Compressive strength f_c'
  - 12" x 6" cylinder
  - 28 day moist cure
  - Ultimate (failure) strength
  - Usable strain ε_{cu} = 0.003 (ACI 318)

- Tensile strength ASTM C496 f_t'
  - 12" x 6" cylinder
  - 28 day moist cure
  - Ultimate (failure) strength
  - Split cylinder test
  - ca. 10% of f'c
  - Neglected in flexure analysis





Failure Modes Based on As

$$\rho = \frac{A_s}{bd}$$ · As · A conc

As,min:
greater of a and b

(a) $\dfrac{3\sqrt{f'_c}}{f_y} b_w d$

(b) $\dfrac{200}{f_y} b_w d$

- **No Reinforcing** BAD
  - Less than As_{min}
  - Brittle failure

- **Reinforcing < balance (use this)** $A_s > As_{min}$
  - Steel yields before concrete fails
  - Ductile failure
  - ($\sim As_{min}$) 0.06 ≥ ε_t ≥ 0.004 ($\sim As_{max}$)
  - ε_t ≥ 0.005 for tension controlled

SAFE
$.75\rho_b$

$$As_{max}\ when\ \varepsilon_t = 0.004$$

- **Reinforcing = balance** ×
  - Concrete fails just as steel yields
  - ε_t at balance = 0.0285
    for Gr 60 ksi steel with 4000 psi concrete

$$\rho_{bal} = \left(\frac{0.85\beta_1 f_c'}{f_y}\right)\left(\frac{87000}{87000 + f_y}\right)$$

- **Reinforcing > balance** LOTS of STEEL
  - Concrete fails before steel yields
  - Low ductility
  - Sudden failure

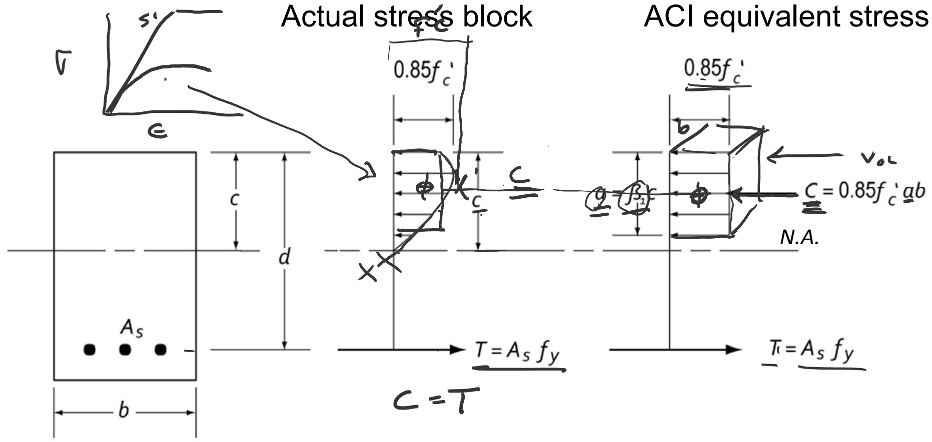
BAD

$$As > As_{max} \quad \text{SuddenDeath!!}$$

ACI Stress Block

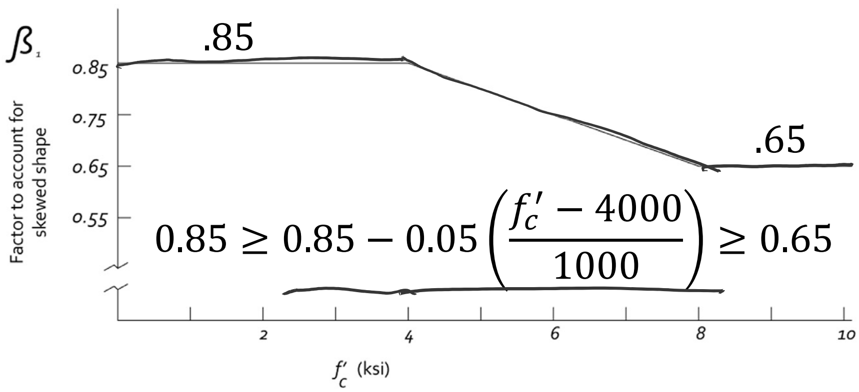
β_1 is a factor to account for the non-linear shape of the compression stress block.

$$a = \beta_1 c$$



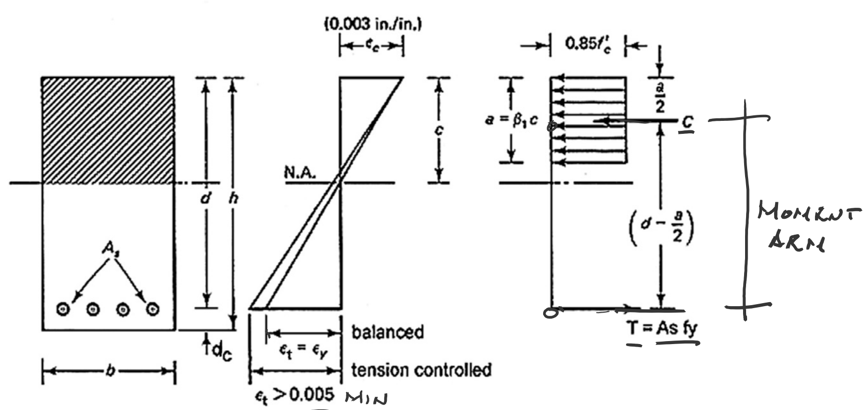
psi

conc. f'_c	β_1
0	0.85
1000	0.85
2000	0.85
3000	0.85
4000	0.85
5000	0.8
6000	0.75
7000	0.7
8000	0.65
9000	0.65
10000	0.65



$$0.85 \geq 0.85 - 0.05\left(\frac{f_c' - 4000}{1000}\right) \geq 0.65$$

Flexure Equations



$\Sigma F_H \rightarrow \quad C = T$

$$0.85 f_c' a b = A_s f_y$$

solving for a,

$$a = \frac{A_s f_y}{0.85 f_c' b} = \frac{\rho f_y d}{0.85 f_c'}$$

STRENGTH

$$M_n = T\left(d - \frac{a}{2}\right) = A_s f_y\left(d - \frac{a}{2}\right)$$

$\frac{wl^2}{8} \rightarrow$ LOAD $M_u \leq \phi M_n$

$$M_u = \phi M_n = \phi A_s f_y\left(d - \frac{a}{2}\right)$$

$$M_u = \phi A_s f_y d\left(1 - 0.59\frac{\rho f_y}{f_c'}\right)$$

$$\boxed{\varepsilon_t = \frac{d - c}{c}(0.003)}$$

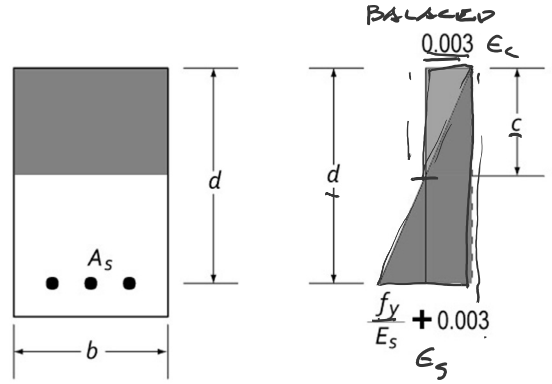
$$\rho = \frac{A_s}{bd}$$

Balance Condition

From similar triangles at balance condition:

$$\frac{c}{d} = \frac{0.003}{0.003 + (f_y/E_s)} = \frac{0.003}{0.003 + (f_y/\underbrace{29 \times 10^6}_{E_s})}$$

$$c = \frac{87{,}000}{87{,}000 + f_y} d$$



Strain diagram for balanced condition.

Use equation for a. Substitute into c = a / β_1

$$a = \frac{\rho f_y d}{0.85 f_c'}$$

$$\rho = \frac{A_s}{bd}$$

$$c = \frac{a}{\beta_1} = \frac{\rho f_y d}{0.85 \beta_1 f_c'}$$

Equate expressions for c:

$c = c$

$$\frac{\rho f_y d}{0.85 \beta_1 f_c'} = \frac{87{,}000}{87{,}000 + f_y} d$$

$$\rho_b = \left(\frac{0.85 \beta_1 f_c'}{f_y}\right)\left(\frac{87{,}000}{87{,}000 + f_y}\right)$$

Table A.8 Balanced Ratio of Reinforcement ρ_b for Rectangular Sections with Tension Reinforcement Only

RE-BARS

f_y \ f_c'		2,500 psi (17.2 MPa) $\beta_1 = 0.85$	3,000 psi (20.7 MPa) $\beta_1 = 0.85$	4,000 psi (27.6 MPa) $\beta_1 = 0.85$	5,000 psi (34.5 MPa) $\beta_1 = 0.80$	6,000 psi (41.4 MPa) $\beta_1 = 0.75$
Grade 40	ρ_b	0.0309	0.0371	0.0495	0.0582	0.0655
40,000 psi	$0.75\rho_b$	0.0232	0.0278	0.0371	0.0437	0.0492
(275.8 MPa)	$0.50\rho_b$	0.0155	0.0186	0.0247	0.0291	0.0328
Grade 50	ρ_b	0.0229	0.0275	0.0367	0.0432	0.0486
50,000 psi	$0.75\rho_b$	0.0172	0.0206	0.0275	0.0324	0.0365
(344.8 MPa)	$0.50\rho_b$	0.0115	0.0138	0.0184	0.0216	0.0243
Grade 60	ρ_b	0.0178	0.0214	0.0285	0.0335	0.0377
60,000 psi	$0.75\rho_b$	0.0134	0.0161	0.0214	0.0252	0.0283
(413.7 MPa)	$0.50\rho_b$	0.0089	0.0107	0.0143	0.0168	0.0189
Grade 75	ρ_b	0.0129	0.0155	0.0207	0.0243	0.0274
75,000 psi	$0.75\rho_b$	0.0097	0.0116	0.0155	0.0182	0.0205
(517.1 MPa)	$0.50\rho_b$	0.0065	0.0078	0.0104	0.0122	0.0137

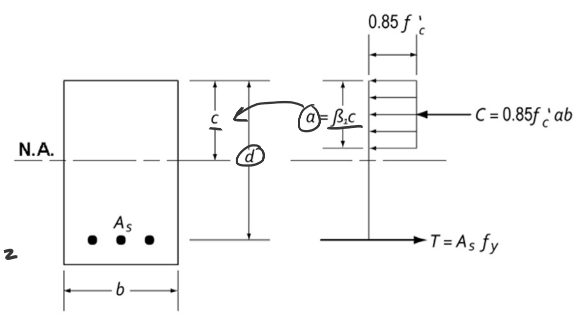
Rectangular Beam Analysis

Data:

- Section dimensions – b, h, (span)
- Steel area - As
- Material properties – f'c, fy

Required:

- Nominal Strength (of beam) Moment - Mn
- Required (by load) Design Moment – Mu $\frac{wl^2}{8}$
- Load capacity



$A_{s\,min}$:
greater of (a) and (b)

(a) $\frac{3\sqrt{f_c'}}{f_y} b_w d$

(b) $\frac{200}{f_y} b_w d$

$$c = \frac{a}{\beta_1}$$

$$\varepsilon_t = \frac{d - c}{c} 0.003 \geq 0.005$$

$$a = \frac{A_s f_y}{0.85 f_c' b}$$

$$M_n = A_s f_y \left(d - \frac{a}{2}\right)$$

$$\varphi M_n \geq M_u \quad (\varphi = 0.9)$$

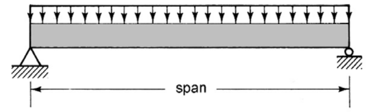
$$\phi M_n = M_u = \frac{(1.2 w_{DL} + 1.6 w_{LL}) l^2}{8}$$

$$1.6 w_{LL} = \frac{M_u 8}{l^2} - 1.2 w_{DL}$$

1. Calculate d
2. Check As min
3. Calculate a
4. Determine c
5. Check that $\varepsilon_t \geq 0.005$ (tension controlled)
6. Find nominal moment, Mn
7. Calculate required moment, ϕ Mn ≥ Mu

   (if $\varepsilon_t \geq 0.005$ then $\phi = 0.9$)
8. Determine max. loading (or span)

Rectangular Beam Analysis

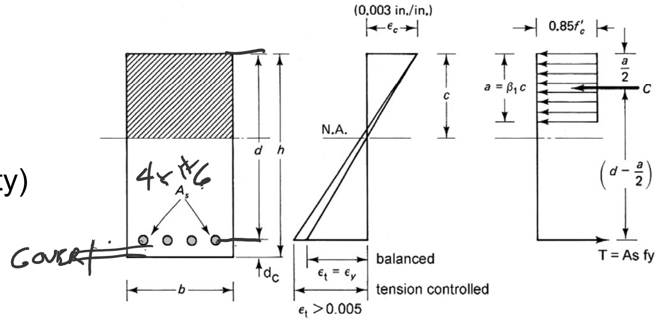


Data:

- Dimensions – 12"x23" h
- Steel – 4 x # 6 fy = 60ksi
- Concrete f'c = 6000 psi
- Stirrup # 3, Cover 1.5"

Required:

- Required Moment – ϕMn = Mu (capacity)

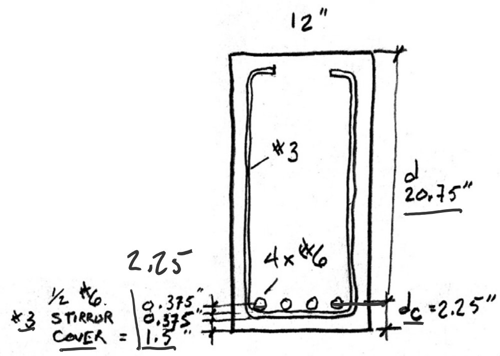


1. Calculate d

d_c = COVER + #3 + ½(#6)

$= 1.5 + 0.375 + \frac{0.75}{2} = 2.25"$

$d = h - d_c = 23" - 2.25" = 20.75"$ d



Rectangular Beam Analysis cont.

Data:
dimensions – 12"x23"
Steel – 4 x # 6 – As = 1.76 in^2
f'c = 6000 psi fy = 60ksi

Table A.2 Designations, Areas, Perimeters, and Weights of Standard Bars

	Customary Units			SI Units		
Bar No.	Diameter (in.)	Cross-sectional Area (in.^{2})	Unit Weight (lb/ft)	Diameter (mm)	Cross-sectional Area (mm^2)	Unit Weight (kg/m)
3	0.375	0.11	0.376	9.52	71	0.560
4	0.500	0.20	0.668	12.70	129	0.994
5	0.625	0.31	1.043	15.88	200	1.552
6	0.750	0.44	1.502	19.05	284	2.235
7	0.875	0.60	2.044	22.22	387	3.042
8	1.000	0.79	2.670	25.40	510	3.973
9	1.128	1.00	3.400	28.65	645	5.060
10	1.270	1.27	4.303	32.26	819	6.404
11	1.410	1.56	5.313	35.81	1006	7.907
14	1.693	2.25	7.650	43.00	1452	11.384
18	2.257	4.00	13.600	57.33	2581	20.238

2. Check As$_{min}$

As min

① $\frac{3\sqrt{f'_c}}{f_y} bd = \frac{3\sqrt{6000}}{60000}(12 \times 20.75) = 0.964 \text{ in}^2$ ← CONTROLS

② $\frac{200\, bd}{f_y} = \frac{200(12)(20.75)}{60000} = 0.83 \text{ in}^2$

$\therefore A_{s\,min} = 0.964 \text{ in}^2$

$A_s = A_b\,(N.B.) = 0.44(4) = 1.76 \text{ in}^2 \checkmark > 0.964 \text{ in}^2$

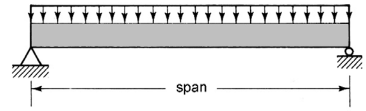
Rectangular Beam Analysis cont.

Data:

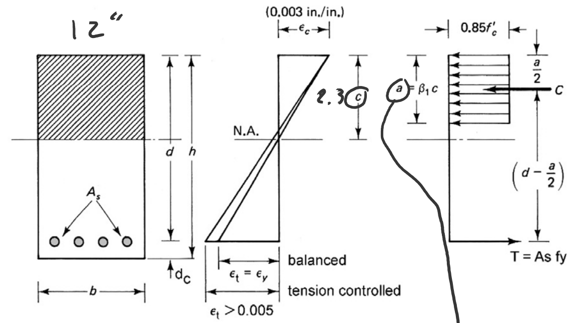
dimensions – 12”x23”

Steel – 4 x # 6 – As = 1.76 in^2

f’c = 6000 psi fy = 60ksi



f'c	β_1
0	0.85
1000	0.85
2000	0.85
3000	0.85
4000	0.85
5000	0.8
6000	0.75
7000	0.7
8000	0.65
9000	0.65
10000	0.65



Beta1:

$$0.85 \geq 0.85 - 0.05\left(\frac{f_c' - 4000}{1000}\right) \geq 0.65$$

3. Find a

$$a = \frac{A_s f_y}{0.85 f'_c b} = \frac{(1.76)(60)}{.85(6)(12)} = 1.725''$$

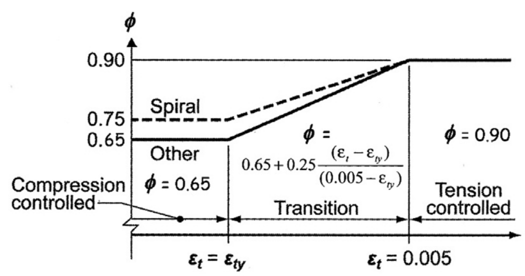
4. Find c

$$\beta_1 = 0.85 - 0.05\,\frac{f'_c - 4000}{1000} = 0.85 - 0.1 = 0.75$$

$$c = a/\beta_1 = \frac{1.725}{0.75} = 2.300''$$

Rectangular Beam Analysis cont.

$$\varepsilon_t = \frac{d-c}{c}0.003 \geq 0.005$$



5. Check that $\varepsilon_t \geq 0.005$
(for tension controlled section)
ϕ = 0.9

$$\epsilon_t = \frac{d-c}{c}\,0.003 = \frac{20.75'' - 2.3''}{2.3''}\,0.003$$

$$= 0.02406 > 0.004 \;\therefore\; \text{ok}$$

$$= 0.02406 > 0.005 \;\therefore\; \text{tension controlled}$$

6. Find nominal moment, Mn

$$T = A_s f_y = 1.76^{in^2}(60^{ksi}) = 105.6\ K$$

$$M_n = T\left(d - \frac{a}{2}\right) = 105.6\left(20.75 - \frac{1.725''}{2}\right)$$

$$M_n = 2100\ K\text{-}''$$

7. Calculate required moment
ϕMn ≥ Mu

$$\overset{0.9}{\phi} M_n = 0.9(2100) = 1890\ K\text{-}''$$

$$w_u = \frac{M_u^{K\text{-}'}\,8}{l^{2\,\prime}} \qquad \frac{w_u l^2}{8} = M_u = \phi M_n = 1890/12 = 157.5\ K\text{-}'$$