Architecture 324
Structures II

Masonry

- TMS 402
- Rational Approach
- Empirical Approach

Chilehaus, Hamburg
Arch: Fritz Höger, 1924

Mortar Types

Types M, S, N, O

The following mortar designations took effect in the mid–1950's:

M a **S** o **N** w **O** r **K**

strongest weakest

Table 2-3. Guide to the Selection of Mortar Type*

Location	Building segment	Mortar type: Recommended	Mortar type: Alternative
Exterior, above grade	Load-bearing walls	N	S or M
	Non-load-bearing walls	O**	N or S
	Parapet walls	N	S
Exterior, at or below grade	Foundation walls, retaining walls, manholes, sewers, pavements, walks, and patios	S†	M or N†
Interior	Load-bearing walls	N	S or M
	Non-load-bearing partitions	O	N

*Adapted from ASTM C270. This table does not provide for specialized mortar uses, such as chimney, reinforced masonry, and acid-resistant mortars.
**Type O mortar is recommended for use where the masonry is unlikely to be frozen when saturated or unlikely to be subjected to high winds or other significant lateral loads. Type N or S mortar should be used in other cases.
†Masonry exposed to weather in a nominally horizontal surface is extremely vulnerable to weathering. Mortar for such masonry should be selected with due caution.
Note: For tuckpointing mortar, see "Tuckpointing," Chapter 9.

Relative Parts by Volume

mortar type	Portland cement	lime	sand
M	1	$\frac{1}{4}$	$3\frac{1}{2}$
S	1	$\frac{1}{2}$	$4\frac{1}{2}$
N	1	1	6
O	1	2	9

sum should equal 1/3 of sand volume
(assuming that sand has void ratio of 1 in 3)

Mortar Types

Type M, S, N, O

Slump is higher than cast concrete based on workability

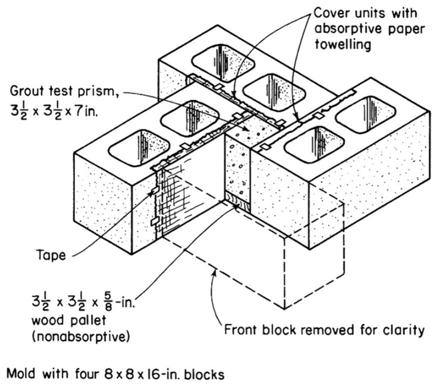


Fig. 2-29. ASTM C1019 method of using masonry units to form a prism for compression-testing of masonry grout.

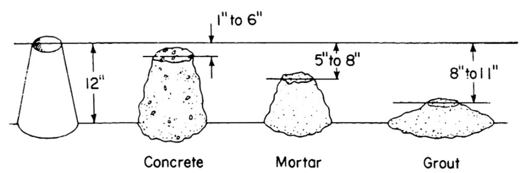
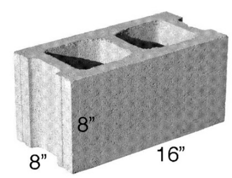


Fig. 2-27. Slump test comparison of concrete, mortar, and masonry grout.

Masonry Strength

Masonry strength, f'm, based on unit strength, fu, and mortar type

Clay Masonry



Concrete Masonry

Required Net Area Compressive Strength of Clay Masonry Units (psi) fu		f'm For Net Area Compressive Strength of Masonry (psi)
When Used With Type M or S Mortar	When Used With Type N Mortar	
1,700	2,100	1,000
3,350	4,150	1,500
4,950	6,200	2,000
6,600	8,250	2,500
8,250	10,300	3,000
9,900	––	3,500
11,500	––	4,000

(*From Masonry Standards Joint Committee* Specifications for Masonry Structures*, ACI 530.1/ASCE 6/TMS 602-99*)

Required Net Area Compressive Strength of Concrete Masonry Units (psi) fu		f'm For Net Area Compressive Strength of Masonry (psi)
When Used With Type M or S Mortar	When Used With Type N Mortar	
1,250	1,300	1,000
1,900	2,150	1,500
2,800	3,050	2,000
3,750	4,050	2,500
4,800	5,250	3,000

(*From* International Building Code 2000 *and Masonry Standards Joint Committee* Specifications for Masonry Structures*, ACI 530.1/ASCE 6/TMS 602-99*)

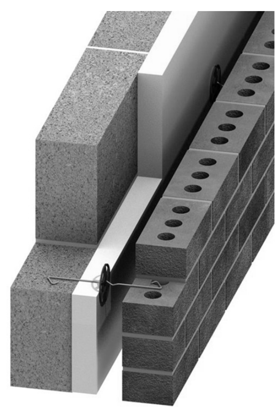
Constructive Properties

Typical Values

Property		Clay Masonry	Concrete Masonry
Unit strength		$8000\ psi$	$2000\ psi$
Type N mortar	f'_m	$2440\ psi$	$1750\ psi$
	E_m	$1.70x10^6\ psi$	$1.58x10^6\ psi$
Type M or S mortar	f'_m	$2920\ psi$	$2000\ psi$
	E_m	$2.05x10^6\ psi$	$1.80x10^6\ psi$

Property	Clay Masonry	Concrete Masonry
Modulus of Elasticity, E_m	$700f'_m$	$900f'_m$
Shear Modulus, G	$0.4E_m$	$0.4E_m$
Coefficient of Creep	$\frac{0.7\ x10^{-7}}{psi}$	$\frac{2.5x10^{-7}}{psi}$

Analysis and Design



Empirical approach
- based on experience
- limits on lateral loading
- limits on height
- limits on eccentricity
- (basically, no flexure)
- non-reinforced

Rational approach
- based on Strength Design (LRFD)
- either reinforced or non-reinforced
- limited by strength

Rational Masonry Analysis

Procedure

Strength Design (LRFD) – **non-reinforced**

Rational Approach

for axial compression using TMS 402 (2016)

Given: geometry, material

Find: axial compressive load capacity, Pn

1. Determine the masonry strength, f'm, based on unit strength, fu, and mortar type (table)
2. Find the net area, A_n, and Moment of Inertia, I_n (see NCMA TEK 14-1B with HW problem pdf.)
3. Calculate radius of gyration, $r = \sqrt{I/A}$
4. Calculate h/r
5. Choose the axial strength equation, Pn:
   If $h/r < 99$ use TMS 402 eq.9-11
   If $h/r > 99$ use TMS 402 eq.9-12
6. Calculate øPn where ø for axial force = 0.90
7. Check that øPn is greater than Pu.

(Equation 9-11) for h/r < 99

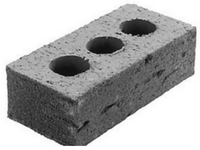
$$P_n = 0.80\left\{0.80 A_n f'_m \left[1-\left(\frac{h}{140r}\right)^2\right]\right\}$$

(Equation 9-12) for h/r > 99

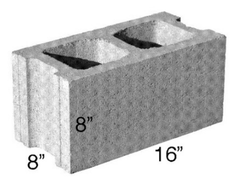
$$P_n = 0.80\left[0.80 A_n f'_m \left(\frac{70r}{h}\right)^2\right]$$

Masonry Strength

Masonry strength, f'm, based on unit strength, fu, and mortar type



Clay Masonry



Concrete Masonry

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Rational Masonry Analysis
Procedure
Strength Design – **non-reinforced**

Rational Approach
for axial compression
using TMS 402 (2016)

Section Properties of Concrete Masonry Walls NCMA TEK 14 – 1B
(attached to problem description, and also on Canvas, and on NCMA website)

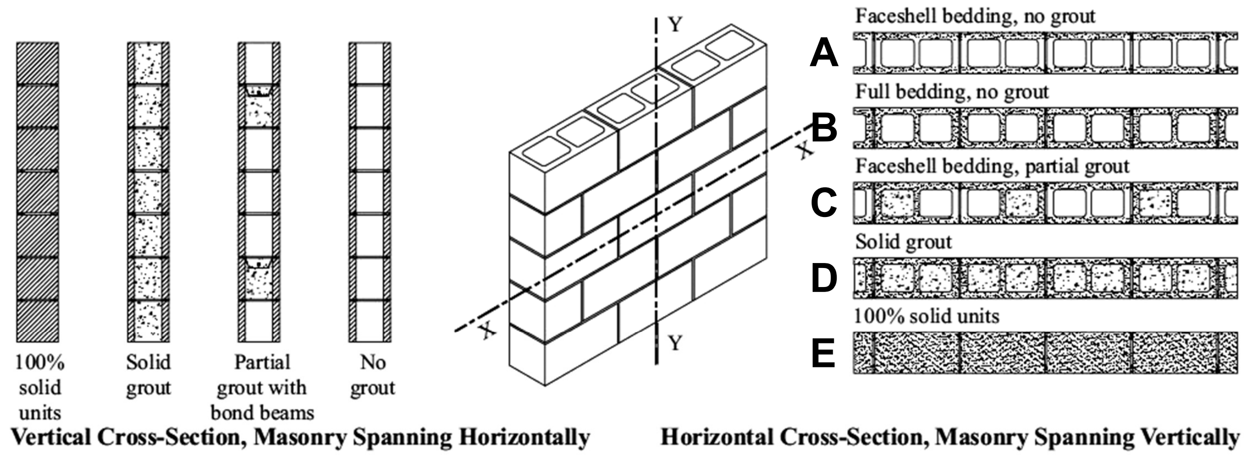


Figure 3—Horizontal and Vertical Cross-Sections

Rational Masonry Analysis
Procedure
Strength Design – **non-reinforced**

Rational Approach
for axial compression
using TMS 402 (2016)

Section Properties of Concrete Masonry Walls NCMA TEK 14 – 1B
(attached to problem description and also on Canvas and on NCMA website)

Table 3—8-inch (203-mm) Single Wythe Walls, 1 1/4 in. (32 mm) Face Shells (standard)

3a: Horizontal Section Properties (Masonry Spanning Vertically)

	Unit	Grout spacing (in.)	Mortar bedding	Net cross-sectional properties[A] A_n (in.^{2}/ft)	I_n (in.^{4}/ft)	S_n (in.^{3}/ft)
A	Hollow	No grout	Face shell	30.0	308.7	81.0
B	Hollow	No grout	Full	41.5	334.0	87.6
D/E	100% solid/solidly grouted		Full	91.5	443.3	116.3
C	Hollow	16	Face shell	62.0	378.6	99.3
↓	Hollow	24	Face shell	51.3	355.3	93.2
	Hollow	32	Face shell	46.0	343.7	90.1
	Hollow	40	Face shell	42.8	336.7	88.3
	Hollow	48	Face shell	40.7	332.0	87.1
	Hollow	72	Face shell	37.1	324.3	85.0
	Hollow	96	Face shell	35.3	320.4	84.0
	Hollow	120	Face shell	34.3	318.0	83.4

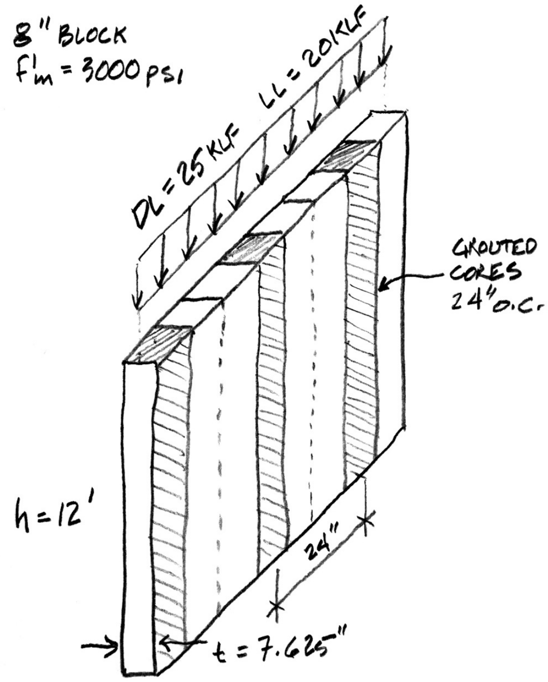
Rational Masonry Analysis
Example
Strength Design – **non-reinforced**

Rational Approach
for axial compression
using TMS 402 (2016)

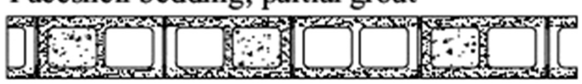
Example Problem

Given: geometry: 8” block, grouted 24” o.c.
material: f’m = 3000 psi
Find: check pass/fail for the given loading

1. Determine the masonry strength, f’m, based on unit strength, fu, and mortar type. (given f’m = 3000 psi)



Faceshell bedding, partial grout



Rational Masonry Analysis
Example
Strength Design – **non-reinforced**

Rational Approach
for axial compression
using TMS 402 (2016)

2. Find the net area, A_n, and Moment of Inertia, I_n (see NCMA TEK 14-1B)

Table 3—8-inch (203-mm) Single Wythe Walls, 1¼ in. (32 mm) Face Shells (standard)

3a: Horizontal Section Properties (Masonry Spanning Vertically)

Unit	Grout spacing (in.)	Mortar bedding	Net cross-sectional properties[A] A_n (in.²/ft)	I_n (in.⁴/ft)	S_n (in.³/ft)
Hollow	No grout	Face shell	30.0	308.7	81.0
Hollow	No grout	Full	41.5	334.0	87.6
100% solid/solidly grouted		Full	91.5	443.3	116.3
Hollow	16	Face shell	62.0	378.6	99.3
Hollow	24	Face shell	51.3	355.3	93.2
Hollow	32	Face shell	46.0	343.7	90.1
Hollow	40	Face shell	42.8	336.7	88.3
Hollow	48	Face shell	40.7	332.0	87.1
Hollow	72	Face shell	37.1	324.3	85.0
Hollow	96	Face shell	35.3	320.4	84.0
Hollow	120	Face shell	34.3	318.0	83.4

Rational Masonry Analysis

Example

Strength Design – **non-reinforced**

Rational Approach

for axial compression using TMS 402 (2016)

3. Calculate $r = \sqrt{I/A}$

TEK 14-1B 8" SINGLE WYTHE

HOLLOW BLOCK – GROUT @ 24" o.c. – FACE SHELL MORTAR

$A_n = 51.3 \text{ in}^2 \quad I_n = 355.3 \text{ in}^4$ (NET)

4. Calculate h/r

$$r = \sqrt{\frac{I}{A}} = \sqrt{\frac{355.3}{51.3}} = 1.952 \text{ in}$$

$$\frac{h}{r} = \frac{12'(12)}{1.952} = 73.75 < 99 \quad \therefore \text{ EQ 9-11}$$

5. Choose the axial strength equation, Pn:
   If h/r < 99 use TMS 402 eq.9-11
   If h/r > 99 use TMS 402 eq.9-12

(Equation 9-11) for h/r < 99

$$P_n = 0.80\left\{0.80 A_n f'_m \left[1 - \left(\frac{h}{140r}\right)^2\right]\right\}$$

Rational Masonry Analysis

Example

Strength Design – **non-reinforced**

Rational Approach

for axial compression using TMS 402 (2016)

Given: geometry: 8" block, grouted 24" o.c.
material: f'm = 3000 psi
Area An = 51.3 in^2/ft
height h = 12 ft
r = 1.952 in

(Equation 9-11) for h/r < 99

$$P_n = 0.80\left\{0.80 A_n f'_m \left[1 - \left(\frac{h}{140r}\right)^2\right]\right\}$$

6. Calculate øPn
   where ø for axial force = 0.90

$$P_n = 0.8\left[0.8\, A_n f'_m \left(1 - \left(\frac{h}{140r}\right)^2\right)\right]$$

$$P_n = 0.8\left[0.8\,(51.3)(3)\left(1 - \left(\frac{144''}{140(1.952'')}\right)^2\right)\right] \quad (\text{in}^2/\text{FT}, \text{ KSI})$$

$$P_n = 0.8\left[123.12 - 0.7223\right] = 71.4 \text{ K/FT}$$

$$\phi P_n = 0.9(71.4) = 64 \text{ K/FT}$$

7. Check that øPn is greater than Pu.

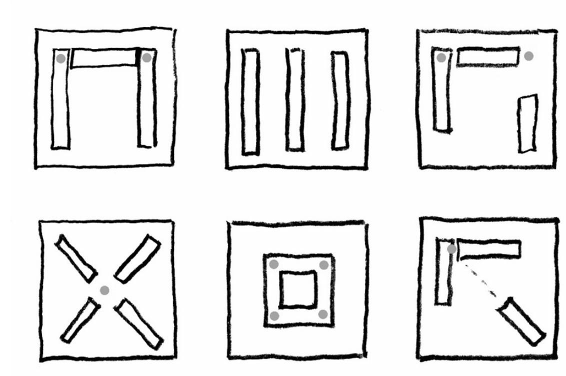
$$P_U = 1.2(25) + 1.6(20) = 62 \text{ K/FT}$$

$$P_U = 62 \text{ K/FT} < 64 \text{ K/FT} = \phi P_n \quad \therefore \text{ OK} ✓$$

Lateral Force Resistance

Stability requires at least 2 points of intersection.

Force is more evenly resisted with centroid of walls in the kern of slab



Empirical Approach

TMS 402-16 Tab. CC A.1.1
Checklist for use of empirical design

COMMENTARY

Table CC-A.1.1 — Checklist for use of Appendix A – Empirical Design of Masonry

1.	Risk Category IV structures, or portions thereof, are not permitted to be designed using Appendix A.
2.	Partitions are not permitted to be designed using Appendix A.
3.	Use of empirical design is limited based on Seismic Design Category, as described in the following table.
	Seismic Design Category / Participating Walls / Non-Participating Walls, except partition walls
	A / Allowed by Appendix A / Allowed by Appendix A
	B / Not Allowed / Allowed by Appendix A
	C / Not Allowed / With prescriptive reinforcement per 7.4.3.1[1]
	D, E, and F / Not Allowed / Not Allowed
	[1] Lap splices are required to be designed and detailed in accordance with the requirements of Chapters 8 or 9.
4.	Use of empirical design is limited based on wind speed at the project site, as described in Code A.1.2.3 and Code Table A.1.1.
5.	If wind uplift on roofs result in net tension, empirical design is not permitted (A.8.3.1).
6.	Loads used in the design of masonry must be listed on the design drawings (1.2.1b).
7.	Details of anchorage to structural frames must be included in the design drawings (1.2.1e).
8.	The design is required to include provisions for volume change (1.2.1h). The design drawings are required to include the locations and sizing of expansion, control, and isolation joints.
9.	If walls are connected to structural frames, the connections and walls are required to be designed to resist the interconnecting forces and to accommodate deflections (4.4). This provision requires a lateral load and uplift analysis for exterior walls that receive wind load and are supported by or are supporting a frame or roofing system.
10.	Masonry not laid in running bond (for example, stack bond masonry) is required to have horizontal reinforcement (4.5).
11.	A project quality assurance plan is required (3.1) with minimum requirements given in TMS 602 Tables 3 and 4 for Quality Assurance Level 1.
12.	The resultant of gravity loads must be determined and assured to be located within certain limitations for walls and piers (A.1.2.1).
13.	Ensure compliance of the design with prescriptive floor, roof, and wall-to-structural framing anchorage requirements, as well as other anchorage requirements (A.8.3 and A.8.4).
14.	Type N mortar is not permitted for foundation walls (A.6.3.1(g)).
15.	Design shear wall lengths, spacings, and orientations to meet the requirements of Code A.3.1.

foundation:

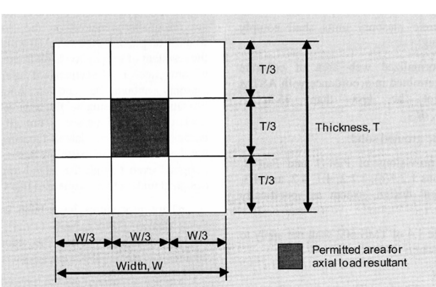


Figure CC-A.1-1 - Area for gravity loads applied to foundation piers

TMS 402 - 2016

Empirical Approach

Risk Catagories:
ASCE – 7

category IV is “not permitted”
with empirical approach

Table 1.5-1 Risk Category of Buildings and Other Structures for Flood, Wind, Snow, Earthquake, and Ice Loads

Use or Occupancy of Buildings and Structures	Risk Category
Buildings and other structures that represent low risk to human life in the event of failure	I
All buildings and other structures except those listed in Risk Categories I, III, and IV	II
Buildings and other structures, the failure of which could pose a substantial risk to human life	III
Buildings and other structures, not included in Risk Category IV, with potential to cause a substantial economic impact and/or mass disruption of day-to-day civilian life in the event of failure	
Buildings and other structures not included in Risk Category IV (including, but not limited to, facilities that manufacture, process, handle, store, use, or dispose of such substances as hazardous fuels, hazardous chemicals, hazardous waste, or explosives) containing toxic or explosive substances where the quantity of the material exceeds a threshold quantity established by the Authority Having Jurisdiction and is sufficient to pose a threat to the public if released[a]	
Buildings and other structures designated as essential facilities	IV
Buildings and other structures, the failure of which could pose a substantial hazard to the community	
Buildings and other structures (including, but not limited to, facilities that manufacture, process, handle, store, use, or dispose of such substances as hazardous fuels, hazardous chemicals, or hazardous waste) containing sufficient quantities of highly toxic substances where the quantity of the material exceeds a threshold quantity established by the Authority Having Jurisdiction and is sufficient to pose a threat to the public if released[a]	
Buildings and other structures required to maintain the functionality of other Risk Category IV structures	

Empirical Approach

Wind limitations:
Basic wind speed ≤ 115 mph
(see TMS 402-16 Tab. A.1.1)

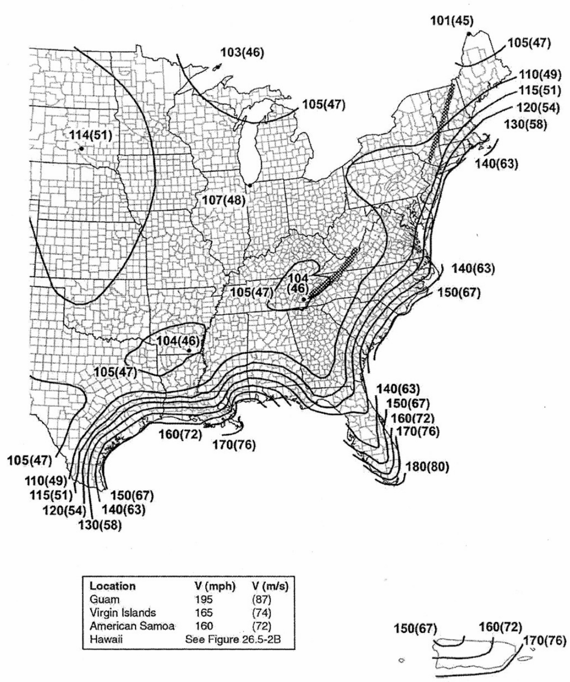
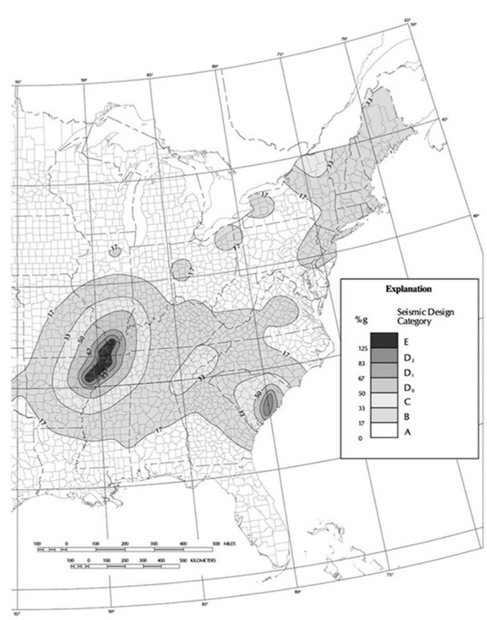


FIGURE 26.5-1B (*Continued*). Basic Wind Speeds for Risk Category II Buildings and Other Structures

ASCE 7 – 2016 basic wind speeds for risk cat. II

Seismic limitations:
Can generally be used for Seismic Design Category (SDC) A, B, or C , or only A if part of the seismic lateral force resisting system.



Seismic zones A -E

Empirical Design of Masonry
TMS 402-16

Height limits by wind speed and application

Table A.1.1 Limitations based on building height and basic wind speed

Element Description	Building Height, ft (m)	Basic Wind Speed, mph (mps)[1] Less than or equal to 115 (51)	Over 115 (51) and less than or equal to-120 (54)	Over 120 (54) and less than or equal to 125 (56)	Over 125 (56)
Masonry elements that are part of the lateral-force-resisting system	35 (11) and less	Permitted			Not Permitted
Interior masonry loadbearing elements that are not part of the lateral-force-resisting system in buildings other than enclosed as defined by ASCE 7	Over 180 (55)	Not Permitted			
	Over 60 (18) and less than or equal to 180 (55)	Permitted	Not Permitted		
	Over 35 (11) and less than or equal to 60 (18)	Permitted		Not Permitted	
	35 (11) and less	Permitted			Not Permitted
Exterior masonry elements that are not part of the lateral-force-resisting system	Over 180 (55)	Not Permitted			
	Over 60 (18) and less than or equal to 180 (55)	Permitted	Not Permitted		
	Over 35 (11) and less than or equal to 60 (18)	Permitted		Not Permitted	
Exterior masonry elements	35 (11) and less	Permitted			Not Permitted

[1] Basic wind speed as given in ASCE 7

Empirical Design of Masonry TEK 14-8B (also TMS 402 – Tab. A.5.1)
International Building Code (IBC) Limitations:

1. Lateral support requirements
2. Location of gravity load (in middle 1/3 of wall)
3. Maximum unreinforced spans

Table 2—Wall Lateral Support Requirements (ref. 1)

Construction (unreinforced)	Maximum wall length-to thickness or height-to thickness ratio[A]
Bearing walls	
Solid units or solid grouted	20
All others	18
Nonbearing walls	
Exterior	18
Interior	36
Cantilever walls[B]	
Solid	6
Hollow	4
Parapets (8-in. (203-mm) thick min.)[B]	3

[A] Ratios are determined using nominal dimensions. For multi-wythe walls where wythes are bonded by masonry headers, the thickness is the nominal wall thickness. When multiwythe walls are bonded by metal wall ties, the thickness is taken as the sum of the wythe thicknesses. Note that Reference 6 includes modified requirements for walls with openings.

[B] The ratios are maximum height-to-thickness ratios and do not limit wall length.

Table 3—Maximum Unreinforced Wall Spans, ft (m)[A]

Wall thickness, in. (mm)	6 (152)	8 (203)	10 (254)	12 (305)
Bearing walls				
Solid or solid grouted	10 (3.0)[B]	13.3 (4.1)	16.6 (5.1)	20 (6.1)
All other	9 (2.7)[B]	12 (3.7)	15 (4.5)	18 (5.5)
Nonbearing walls				
Exterior	9 (2.7)	12 (3.7)	15 (4.5)	18 (5.5)
Interior	18 (5.5)	24 (7.3)	30 (9.1)	36 (11)
Cantilever Walls[C]				
Solid	3 (0.9)	4 (1.2)	5 (1.5)	6 (1.8)
Hollow	2 (0.6)	2.6 (0.8)	3.3 (1.0)	4 (1.2)
Parapets [C]	1.5 (0.5)	2 (0.6)	2.5 (0.8)	3 (0.9)

[A] Note that Ref. 6 includes modified requirements for walls with openings.

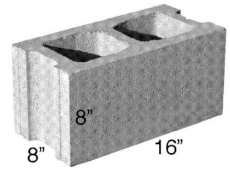
[B] Unreinforced 6-in. (152-mm) thick bearing walls are limited to one story in height.

[C] For these cases, spans are maximum wall heights.

Masonry Strength

Masonry strength, f'm, based on unit strength, fu, and mortar type

Clay Masonry



Concrete Masonry

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4,950	6,200	2,000
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(*From Masonry Standards Joint Committee* Specifications for Masonry Structures, *ACI 530.1/ASCE 6/TMS 602-99*)

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3,750	4,050	2,500
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(*From* International Building Code 2000 *and Masonry Standards Joint Committee* Specifications for Masonry Structures, *ACI 530.1/ASCE 6/TMS 602-99*)

Empirical Design of Masonry TEK 14-8B (also TMS 402 – Tab. A.4.2)

Allowable compressive stress of concrete masonry:

Solid or solidly grouted walls

Table 4—Allowable Compressive Stress for Empirical Design of Masonry

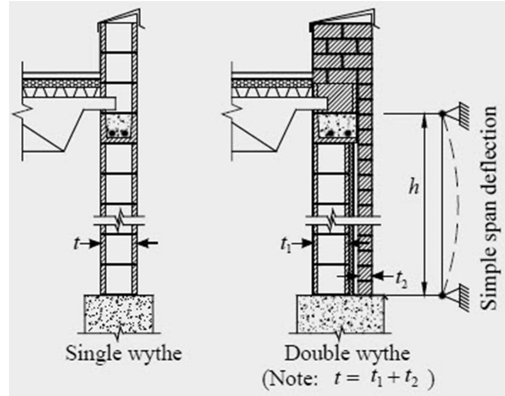
Gross area compressive strength of unit, psi (MPa)	Allowable compressive stresses based on gross cross-sectional area, psi (MPa)[A] Type M or S mortar	Type N mortar
Solid and Solidly Grouted Masonry (refs. 1, 6):		
Solid concrete brick:		
8,000 (55) or greater	350 (2.41)	300 (2.07)
4,500 (31)	225 (1.55)	200 (1.38)
2,500 (17)	160 (1.10)	140 (0.97)
1,500 (10)	115 (0.79)	100 (0.69)
Grouted concrete masonry:		
4,500 (31) or greater	225 (1.55)	200 (1.38)
2,500 (17)	160 (1.10)	140 (0.97)
1,500 (10)	115 (0.79)	100 (0.69)
Solid concrete masonry units:		
3,000 (21) or greater	225 (1.55)	200 (1.38)
2,000 (14)	160 (1.10)	140 (0.97)
1,200 (8.3)	115 (0.79)	100 (0.69)
Hollow walls (noncomposite masonry bonded[B]):		
Solid units:		
2,500 (17) or greater	160 (1.10)	140 (0.97)
1,500 (10)	115 (0.79)	100 (0.69)

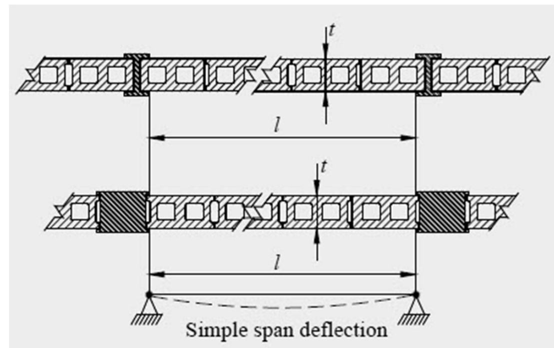
Hollow unit walls

Gross area compressive strength of unit, psi (MPa)	Allowable compressive stresses based on gross cross-sectional area, psi (MPa)[A] Type M or S mortar	Type N mortar
Hollow Unit Masonry (Units Complying With ASTM C 90-06 or Later) (ref. 6)[C]:		
Hollow loadbearing CMU, $t \le 8$ in. (203 mm)[D]:		
2,000 (14) or greater	140 (0.97)	120 (0.83)
1,500 (10)	115 (0.79)	100 (0.69)
1,000 (6.9)	75 (0.52)	70 (0.48)
700 (4.8)	60 (0.41)	55 (0.38)
Hollow loadbearing CMU, 8 in. $< t <$ 12 in. (203 to 305 mm)[D]:		
2,000 (14) or greater	125 (0.86)	110 (0.76)
1,500 (10)	105 (0.72)	90 (0.62)
1,000 (6.9)	65 (0.49)	60 (0.41)
700 (4.8)	55 (0.38)	50 (0.35)
Hollow loadbearing CMU, $t \ge 12$ in (305 mm)[D]:		
2,000 (14) or greater	115 (0.79)	100 (0.69)
1,500 (10)	95 (0.66)	85 (0.59)
1,000 (6.9)	60 (0.41)	55 (0.38)
700 (4.8)	50 (0.35)	45 (0.31)
Hollow walls (noncomposite masonry bonded[B]):		
$t \le 8$ in. (203 mm)[D]	75 (0.52)	70 (0.48)
$8 < t < 12$ in (203 to 305 mm)[D]	70 (0.48)	65 (0.45)
$t \ge 12$ in (305 m.m)[D]	60 (0.41)	55 (0.38)

Empirical Concrete Masonry

Procedure using TMS 402 - 2016





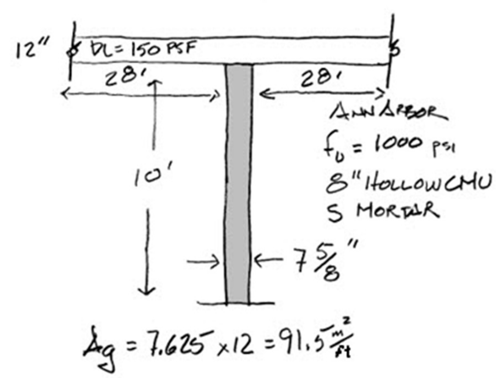
Given: location, geometry, material
Find: strength (load capacity)

1. Check axial loading – must be within middle 1/3
2. Check seismic category to be A, B, or C , or only A if part of the seismic lateral force resisting system.
3. Check wind speed (ASCE-7 2016) compare with Tab. A.1.1
4. Check minimum thickness.
   1 story = 6" min.   2 story = 8" min.
5. Check lateral support (vertical or horizontal) tables 2 and 3 TEK 14-8B or TMS 402 – Tab. A.5.1
6. Determine allowable compressive stress from table 4 TEK 14-8B or TMS 402 – Tab. A.4.2
7. Allowable load = (stress) (gross area) (not LRFD so no γ factors)

$$P = F \times A_g$$

Empirical Design Example

Given:
8" hollow non-reinforced CMU wall
interior wall, Ann Arbor, Mich.
DL = 150 psf



Find:
LL capacity

Checks:

Axially loaded :
loaded within middle 1/3 (kern)

AXIAL LOADING ✓

Seismic Category:
A, B, or C , or only A if part of the seismic lateral force resisting system

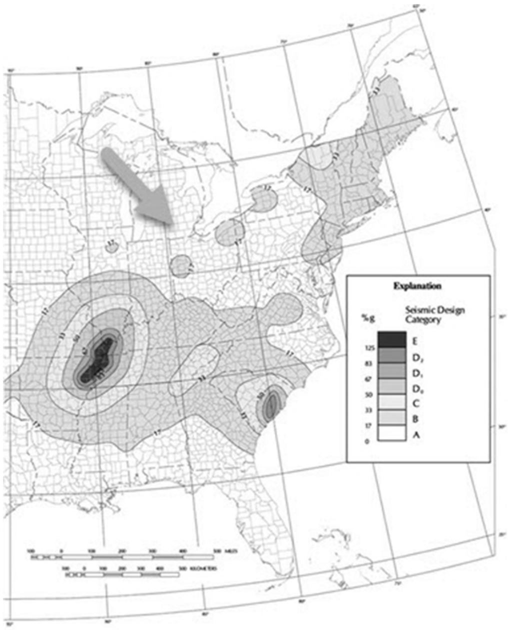
FOR ANN ARBOR :
SDC → A ✓

Wind:
less that 115 mph (ASCE 7 - 2016)

WIND LOAD 107 MPH < 115 ✓

Wind and Seismic Limits

Wind for Ann Arbor – 107 mph
SCD for Ann Arbor - Zones A



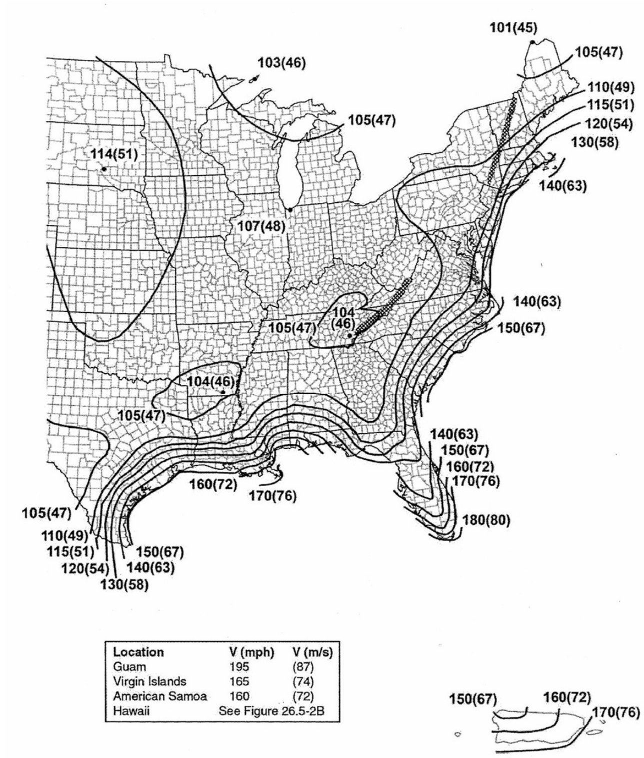


FIGURE 26.5-1B (*Continued*). Basic Wind Speeds for Risk Category II Buildings and Other Structures

Empirical Design Example

Checks:

Maximum height – Table A.1.1

- wind speed = 107 mph
- interior, loadbearing
- h < 35 ft

MAX HEIGHT
TABLE 1 10' ✓

H/t (TABLE 2)
120" / 8" = 15 < 18 ✓

MAX. UNREINF. HEIGHT
TABLE 3 → 10' < 12' ✓

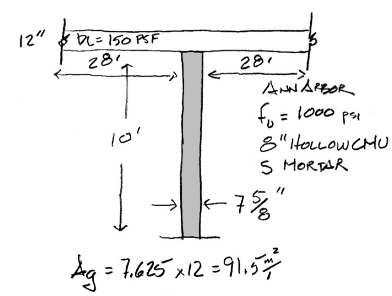


Table A.1.1 Limitations based on building height and basic wind speed

Element Description	Building Height, ft (m)	Basic Wind Speed, mph (mps)[1]: Less than or equal to 115 (51)	Over 115 (51) and less than or equal to-120 (54)	Over 120 (54) and less than or equal to 125 (56)	Over 125 (56)
Masonry elements that are part of the lateral-force-resisting system	35 (11) and less	Permitted			Not Permitted
Interior masonry loadbearing elements that are not part of the lateral-force-resisting system in buildings other than enclosed as defined by ASCE 7	Over 180 (55)	Not Permitted			
	Over 60 (18) and less than or equal to 180 (55)	Permitted	Not Permitted		
	Over 35 (11) and less than or equal to 60 (18)	Permitted		Not Permitted	
	35 (11) and less		Permitted		Not Permitted
Exterior masonry elements that are not part of the lateral-force-resisting system	Over 180 (55)	Not Permitted			
	Over 60 (18) and less than or equal to 180 (55)	Permitted	Not Permitted		
	Over 35 (11) and less than or equal to 60 (18)	Permitted		Not Permitted	
Exterior masonry elements	35 (11) and less	Permitted			Not Permitted

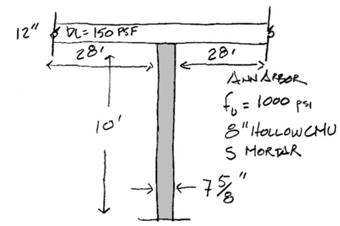
[1] Basic wind speed as given in ASCE 7

Empirical Design Example

MAX HEIGHT
TABLE 1 10' ✓

H/t (TABLE 2)
$\frac{120''}{8''} = 15 < 18$ ✓

MAX. UNREINF. HEIGHT
TABLE 3 → 10' < 12' ✓



$A_g = 7.625 \times 12 = 91.5 \text{ in}^2/1$

Checks:

Minimum bracing – table 2

Maximum unreinforced height - table 3

Table 2—Wall Lateral Support Requirements (ref. 1)

Construction (unreinforced)	Maximum wall length-to thickness or height-to thickness ratio[A]
Bearing walls	
Solid units or solid grouted	20
All others	18
Nonbearing walls	
Exterior	18
Interior	36
Cantilever walls[B]	
Solid	6
Hollow	4
Parapets (8-in. (203-mm) thick min.)[B]	3

Table 3—Maximum Unreinforced Wall Spans, ft (m)[A]

Wall thickness, in. (mm)	6 (152)	8 (203)	10 (254)	12 (305)
Bearing walls				
Solid or solid grouted	10 (3.0)[B]	13.3 (4.1)	16.6 (5.1)	20 (6.1)
All other	9 (2.7)[B]	12 (3.7)	15 (4.5)	18 (5.5)
Nonbearing walls				
Exterior	9 (2.7)	12 (3.7)	15 (4.5)	18 (5.5)
Interior	18 (5.5)	24 (7.3)	30 (9.1)	36 (11)
Cantilever Walls[C]				
Solid	3 (0.9)	4 (1.2)	5 (1.5)	6 (1.8)
Hollow	2 (0.6)	2.6 (0.8)	3.3 (1.0)	4 (1.2)
Parapets[C]	1.5 (0.5)	2 (0.6)	2.5 (0.8)	3 (0.9)

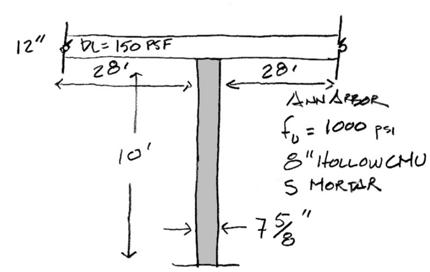
[A] Note that Ref. 6 includes modified requirements for walls with openings.

Empirical Design Example

Find allowable stress – table 4

Find load
P = F Ag

Calculate per foot using gross Area



$A_g = 7.625 \times 12 = 91.5 \text{ in}^2/1$

psi (Mpa)	psi (Mpa)	
Hollow Unit Masonry (Units Complying With ASTM C 90-06 or Later) (ref. 6)[C]:	Type M or S	Type N
Hollow loadbearing CMU, $t \le 8$ in	mortar	mortar
2,000 (14) or greater	140 (0.97)	120 (0.83)
1,500 (10)	115 (0.79)	100 (0.69)
1,000 (6.9)	75 (0.52)	70 (0.48)
700 (4.8)	60 (0.41)	55 (0.38)
Hollow loadbearing CMU, 8 in. < t < 12 in. (203 to 305 mm)[D]:		
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Hollow walls (noncomposite masonry bonded[B]):		
$t \le 8$ in. (203 mm)[D]	75 (0.52)	70 (0.48)
8 < t < 12 in (203 to 305 mm)[D]	70 (0.48)	65 (0.45)
$t \ge 12$ in (305 m.m)[D]	60 (0.41)	55 (0.38)

TABLE 4 HOLLOW 8" f'_0 = 1000
TYPE S → 75 PSI

$P = F A_g = 75\,(7.625 \times 12)$
$= 6862 \text{ \#}/1$

TRIBUTARY STRIP = 28'

$P = 6862 = DL(28') + LL(28')$
$= 150(28) + LL(28)$

LL = 95 PSF CAPACITY