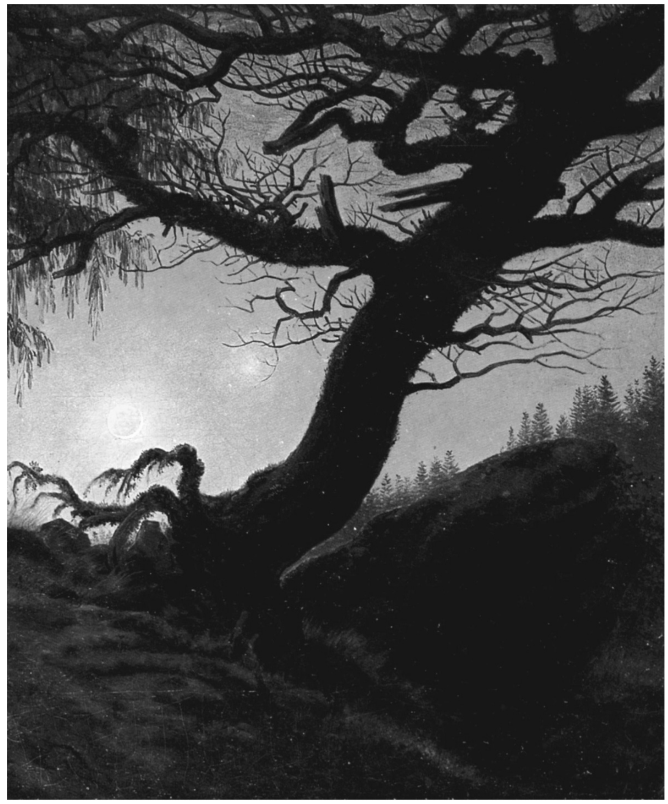
Combined Stress



- Axial vs. Eccentric Load
- Combined Stress
- Interaction Formulas

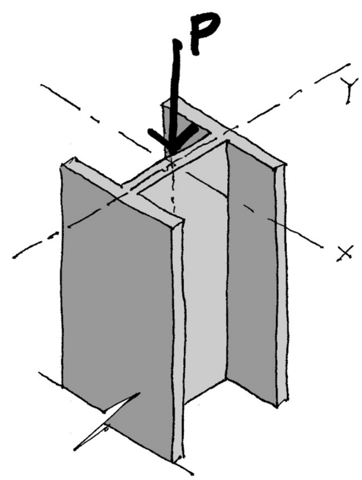
from "Man und Frau den Mond betrachtend"
1830-35 by Caspar David Friedrich
Alte Nationalgalerie, Berlin

Axial Stress

- Loads pass through the centroid of the section , i.e. axially loaded
- Member is straight
- Load less than buckling load

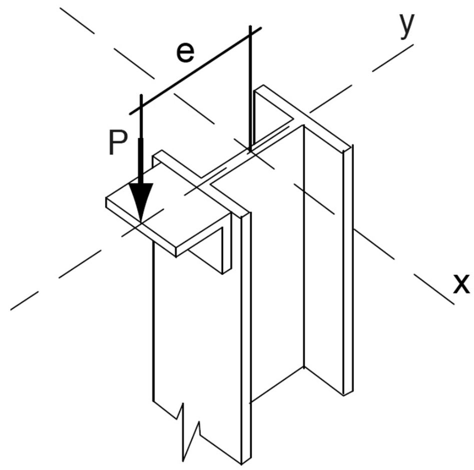
Then:

$$f_a = \frac{P}{A}$$



Eccentric Loads

- Load is offset from centroid
- Bending Moment = P e
- Total load = P + M



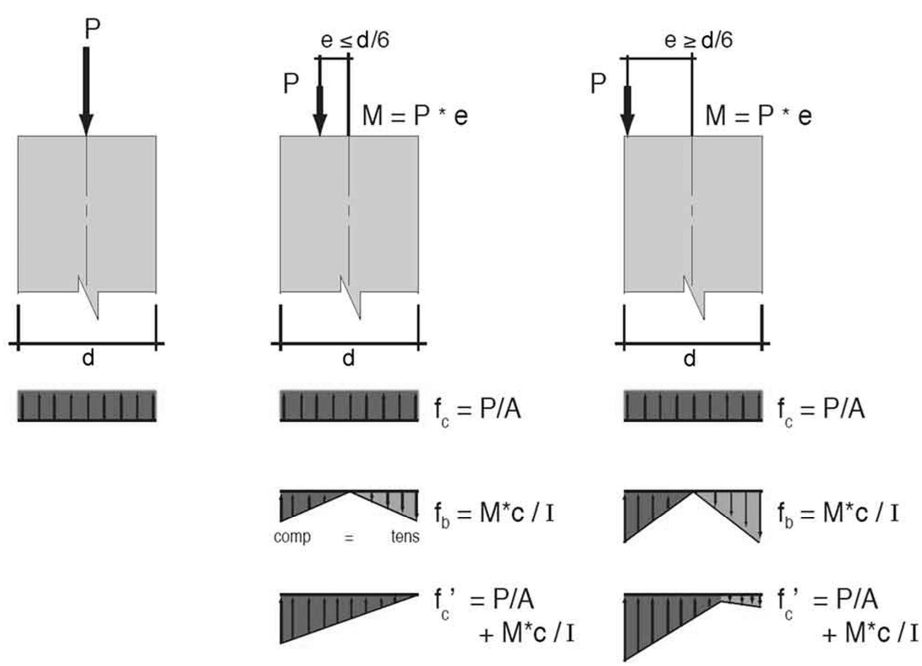
Interaction formula

$$f = \frac{P}{A} \pm \frac{Mc}{I}$$

$$\frac{f_a}{F_a} \pm \frac{f_b}{F_b} \leq 1.0$$

Combined Stress

- Stresses combine by superposition
- Values add or subtract by sign



axial loaded - uniform compressive stress.

small eccentricity - linearly varying stress.

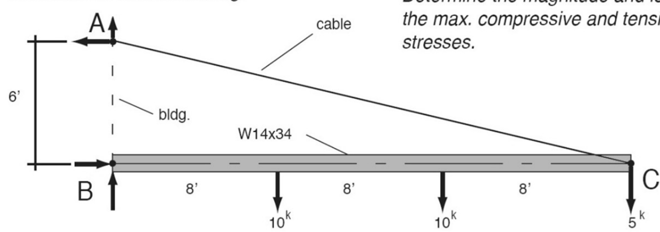
large eccentricity - tensile stress on part of cross section.

Example

CANOPY CONSTRUCTION PROJECTING FROM FACE OF BUILDING.
The supporting cable is pin-connected on the centroidal axis of the steel beam.

Reactions at face of building.

FOR THE W14x34:
Determine the magnitude and location of the max. compressive and tensile unit stresses.



1. Determine external reactions

$\Sigma M_A = 0 = -B_H 6' + 10^k(8') + 10^k(16') + 5^k(24')$

$\underline{B_H = 60^k}$

$\Sigma M_B = 0 = -A_H(6') + 10^k(8') + 10^k(16') + 5^k(24')$

$\underline{A_H = 60^k}$

CHECK $\Sigma F_H = 0 = 60^k - 60^k$ ✓

FBD @ A

A_v, 60^k, 4, 1, 60^k

$\frac{60}{4} : \frac{A_v}{1}$

$\underline{A_v = 15^k}$

$\Sigma F_V = 0 = 15^k - 10^k - 10^k - 5^k + B_V$

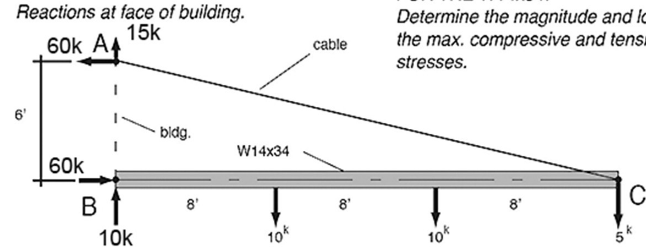
$\underline{B_V = 10^k}$

Example

CANOPY CONSTRUCTION PROJECTING FROM FACE OF BUILDING.
The supporting cable is pin-connected on the centroidal axis of the steel beam.

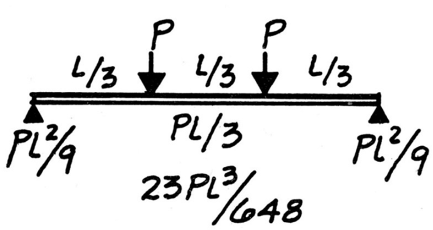
Reactions at face of building.

FOR THE W14x34:
Determine the magnitude and location of the max. compressive and tensile unit stresses.



2. Determine internal member **forces**: Axial and Flexural

3. Determine axial and flexural **stresses**



W14x34 $A = 10.0\ in^2$

$S_x = 48.6\ in^3$

FORCE:

AXIAL $= 60^K$

FLEXURAL $= M = PL/3 = 10^K(8') = 80^{'-K}$

STRESS:

AXIAL $= f_a = \frac{P}{A} = 60^K / 10\,in^2 = \underline{6.0\ KSI}$

FLEXURAL $: f_b = M/S = 80^{'-K}(12) / 48.6\,in^3 = \underline{19.75\ KSI}$

Example

2. Use interaction formula to determine combined stresses at key locations (e.g. extreme fibers)

COMBINED STRESS

TOP SIDE:

$$f_a + f_b = 6.0 + 19.75 = 25.75 \text{ KSI (COMP)}$$

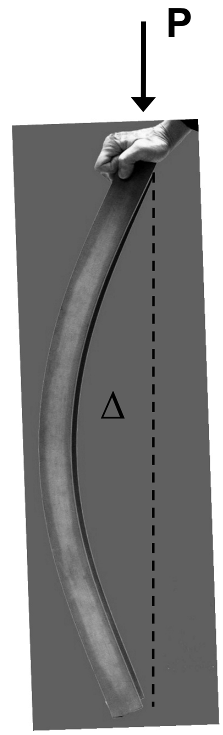
BOTTOM SIDE:

$$f_a - f_b = 6.0 - 19.75 = -13.75 \text{ KSI (TENS)}$$

Second Order Stress
"P Delta Effect"

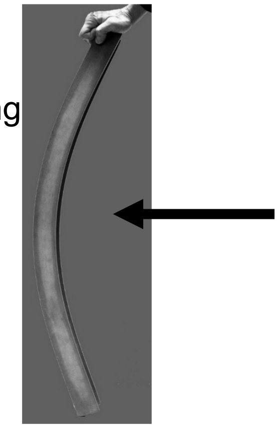
With larger deflections this can become significant.

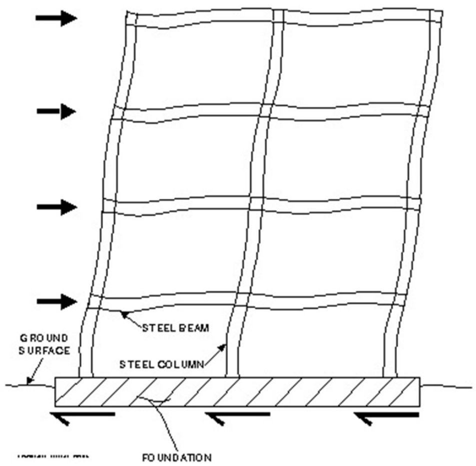
1. Eccentric load causes bending moment
2. Bending moment causes deflection, Δ
3. P x Δ causes additional moment



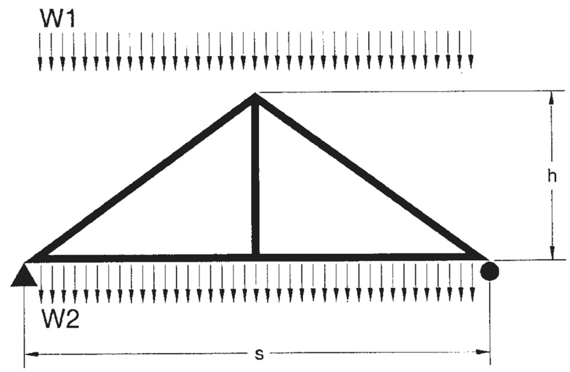
Other Examples of Combined Stress

Columns with side loading



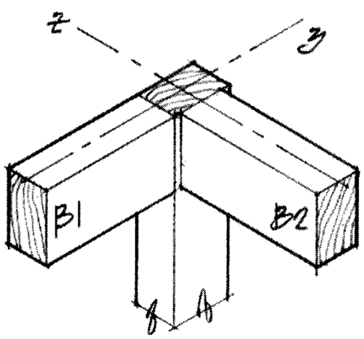


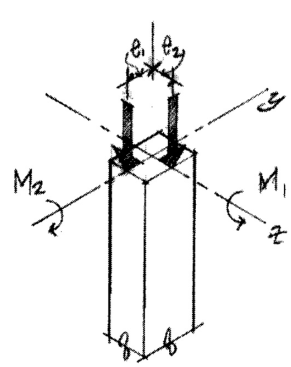
Moment frames



Trusses loaded on members

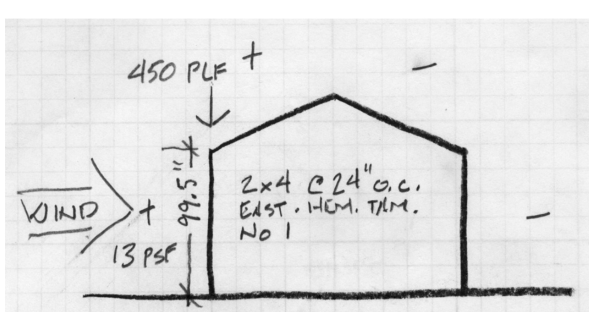
Other Examples of Combined Stress





$M_1 = P_1 \times e_1$ (ABOUT THE x-AXIS)
$M_2 = P_2 \times e_2$ (ABOUT THE y-AXIS)

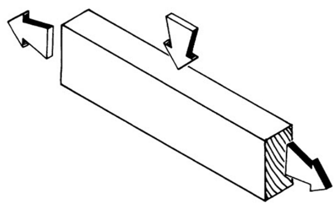
Eccentrically loaded columns



Wind load on walls

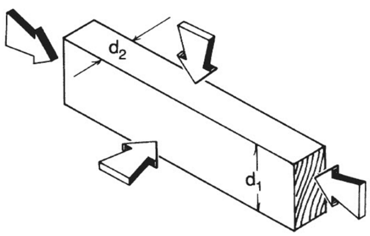
Combined Stress in NDS

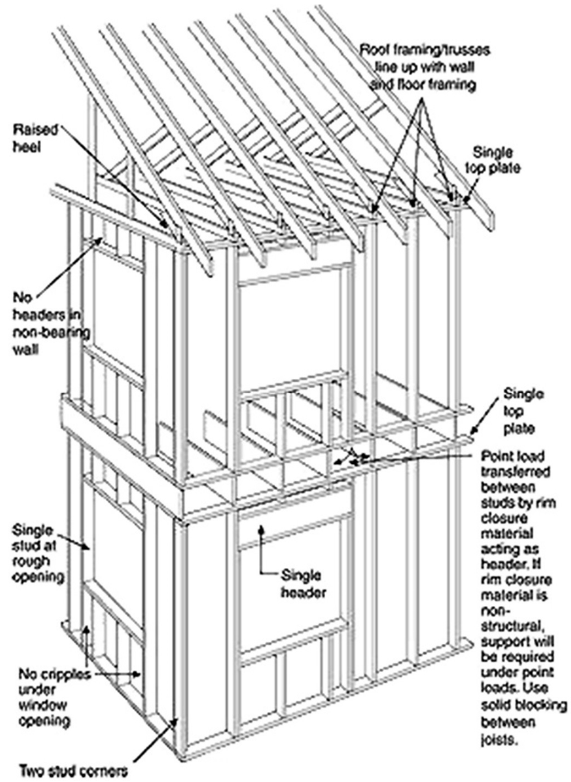
Figure 3G **Combined Bending and Axial Tension**



3.9.2 Bending and Axial Compression

Figure 3H **Combined Bending and Axial Compression**





Tension + Flexure

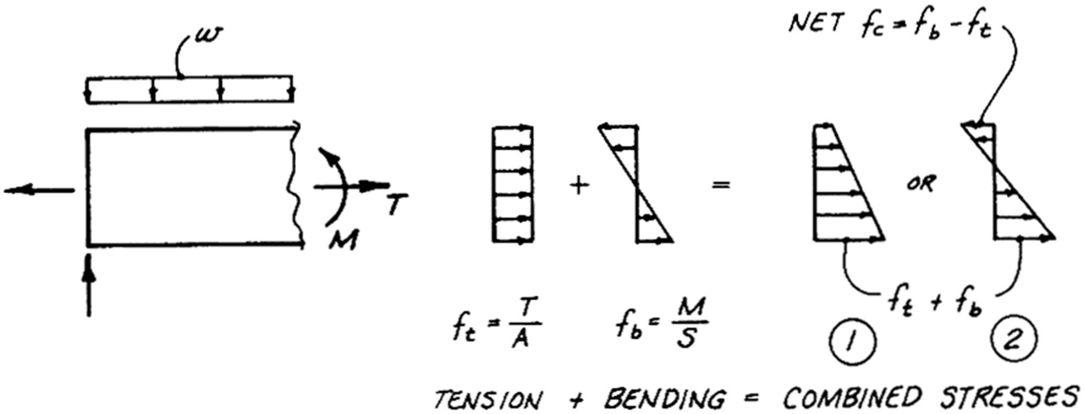
NDS Equations

CASE 1. Tension is critical. Eq. 3.9-1
* no C_L

$$\frac{f_t}{F_{t'}} + \frac{f_b}{F_b *} \leq 1.0$$

CASE 2. Flexure is critical. Eq. 3.9-2
** no C_V

$$\frac{f_b - f_t}{F_b **} \leq 1.0$$



Tension + Flexure

3.9.1 Bending and Axial Tension

Members subjected to a combination of bending and axial tension (see Figure 3G) shall be so proportioned that:

$$\frac{f_t}{F_t'} + \frac{f_b}{F_b^*} \leq 1.0 \qquad \text{TENSION CRIT.} \qquad (3.9\text{-}1)$$

and

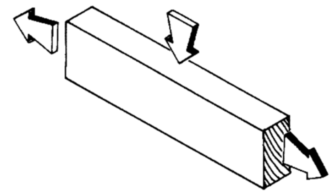
$$\frac{f_b - f_t}{F_b^{**}} \leq 1.0 \qquad \text{FLEXURE CRIT.} \qquad (3.9\text{-}2)$$

where:

F_b^* = reference bending design value multiplied by all applicable adjustment factors except C_L

F_b^{**} = reference bending design value multiplied by all applicable adjustment factors except C_V

Figure 3G Combined Bending and Axial Tension

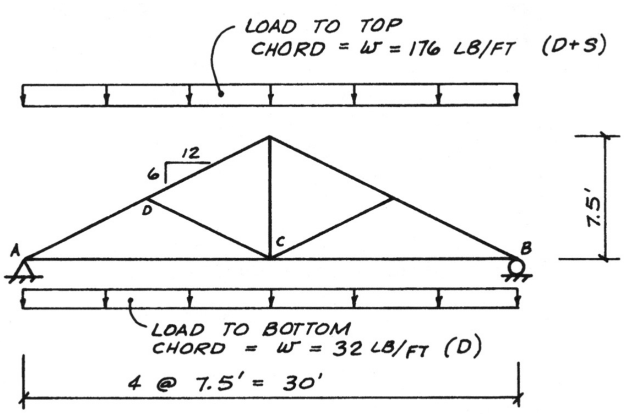


Example Problem

Given: Queen Post truss
- Hem-Fir No.1 & Better
- F_b = 1100 psi
- F_t = 725 psi
- F_c = 1350 psi
- E_{min} = 550000 psi

- span = 30 ft. spaced 48" o.c.
- D + S Load = 44 psf (projected)
- D (attic + ceiling) = 8 psf

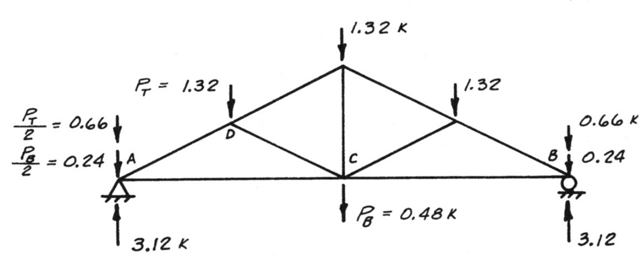
- bottom chord: 2x8
- top chord: 2x10



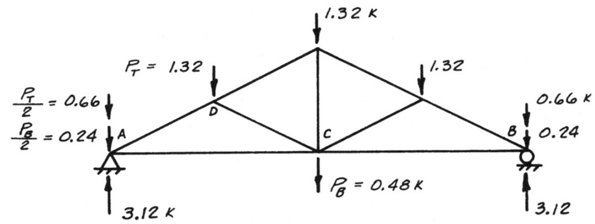
Find: pass/fail

$$\boxed{\frac{f_t}{F_{t'}} + \frac{f_b}{F_b*} \leq 1.0} \qquad \boxed{\frac{f_b - f_t}{F_b**} \leq 1.0}$$

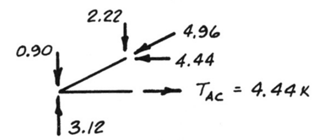
1. Determine truss joint loading



Example (cont.)

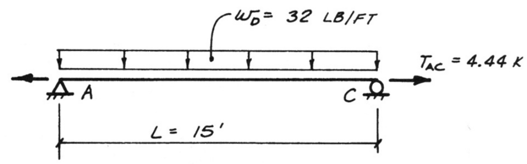


2. Determine the external **end reactions** of the whole truss. The geometry and loads are symmetric, so each reaction is ½ of the total load.



3. Use an FBD of the reaction joint to find the **chord forces**. Sum the forces horizontal and vertical to find the components.

   Top chord = 4.96 k compression
   Bottom chord = 4.44 k tension



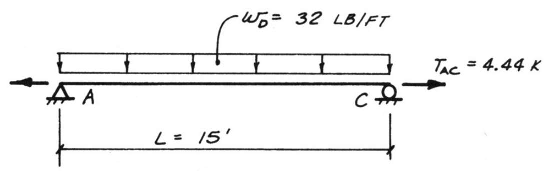
Example

$$\frac{f_t}{F_t'} + \frac{f_b}{F_b^*} \le 1.0$$

and

$$\frac{f_b - f_t}{F_b^{**}} \le 1.0$$

bottom chord 2x8
A = 10.875 in.
S_x = 13.13 in^3



4. Calculate the **actual** axial and flexural stress.

   f_t = 408.3 psi
   f_b = 821.9 psi

$$f_t = \frac{P}{A} = \frac{4440 \text{ lbs}}{10.875 \text{ in}^2} = 408.3 \text{ psi}$$

$$f_b = \frac{M}{S_x} = \frac{900\,(12)}{13.14} = 821.9 \text{ psi}$$

$$M = \frac{w\ell^2}{8} = \frac{32\,(15)^2}{8} = 900 \text{ '-\#}$$

$$S_x = 13.14 \text{ in}^3$$

5. Determine **allowable** stresses using applicable factors:

   (tension: D+S)
   $F_t' = F_t\,(C_D\, C_F)$
   F_t' = 725 (1.15 1.2) = 1000 psi > 408.3

   (flexure: D+S)
   $F_b' = F_b\,(C_D\, C_L\, C_F)$
   F_b' = 1100 (1.15 1.0 1.2) = 1518 psi > 821.9 psi

C_L IS 1.0 BY 4.4.1
d/b < 4 , ENDS ARE HELD

Example

bottom chord 2x8

$$\frac{f_t}{F_t'} + \frac{f_b}{F_b^*} \le 1.0$$

and

$$\frac{f_b - f_t}{F_b^{**}} \le 1.0$$

5. Determine **allowable** stresses using applicable factors:

(tension: D+S)
$F_t' = F_t\,(C_D\ C_F)$
$F_t' = 725\ (1.15\ \ 1.2) = 1000$ psi > 408.3

(flexure: D+S)
$F_b' = F_b\,(C_D\ C_L\ C_F)$
$F_b' = 1100\ (1.15\ 1.0\ 1.2) = 1518$ psi > 821.9 psi

Size Factors, C_F

Grades	Width (depth)	F_b Thickness (breadth) 2" & 3"	F_b Thickness (breadth) 4"	F_t	F_c
Select Structural, No.1 & Btr, No.1, No.2, No.3	2", 3", & 4"	1.5	1.5	1.5	1.15
	5"	1.4	1.4	1.4	1.1
	6"	1.3	1.3	1.3	1.1
	8"	1.2	1.3	1.2	1.05
	10"	1.1	1.2	1.1	1.0
	12"	1.0	1.1	1.0	1.0
	14" & wider	0.9	1.0	0.9	0.9
Stud	2", 3", & 4"	1.1	1.1	1.1	1.05
	5" & 6"	1.0	1.0	1.0	1.0
	8" & wider	Use No.3			
Construction Standard	2", 3", & 4"	1.0	1.0	1.0	1.0
Utility	4"	1.0	1.0	1.0	1.0
	2" & 3"	0.4	—	0.4	0.6

4.4.1 Stability of Bending Members

3.3.3.2 When rectangular sawn lumber bending members are laterally supported in accordance with 4.4.1, $C_L = 1.0$.

4.4.1.2 As an alternative to 4.4.1.1, rectangular sawn lumber beams, rafters, joists, or other bending members, shall be designed in accordance with the following provisions to provide restraint against rotation or lateral displacement. If the depth to breadth, d/b, based on nominal dimensions is:

(a) $d/b \le 2$; no lateral support shall be required.
(b) $2 < d/b \le 4$; the ends shall be held in position, as by full depth solid blocking, bridging, hangers, nailing, or bolting to other framing members, or other acceptable means.

Example

bottom chord 2x8

3.9.1 Bending and Axial Tension

Members subjected to a combination of bending and axial tension (see Figure 3G) shall be so proportioned that:

$$\frac{f_t}{F_t'} + \frac{f_b}{F_b^*} \le 1.0 \qquad \text{TENSION CRIT.} \qquad (3.9\text{-}1)$$

and

$$\frac{f_b - f_t}{F_b^{**}} \le 1.0 \qquad \text{FLEXURE CRIT.} \qquad (3.9\text{-}2)$$

where:

F_b^* = reference bending design value multiplied by all applicable adjustment factors except C_L

F_b^{**} = reference bending design value multiplied by all applicable adjustment factors except C_V

f_t = 408.3 psi f_b = 821.9 psi

F'_t = 1000 psi F'_b = 1518 psi

(3.9-1)

$$\frac{408.3}{1000} + \frac{821.9}{1518}$$

$0.4083 + 0.5414 = 0.95$

$0.95 < 1.0$ ✓PASS

(3.9-2)

$$\frac{821.9 - 408.3}{1518} = 0.2724$$

$0.27 < 1.0$ ✓PASS

Bending + Axial Compression

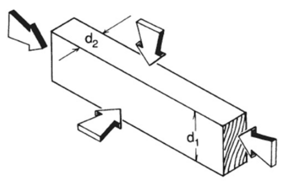
3.9.2 Bending and Axial Compression

Members subjected to a combination of bending about one or both principal axes and axial compression (see Figure 3H) shall be so proportioned that:

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]} + \frac{f_{b2}}{F_{b2}'\left[1-\left(f_c/F_{cE2}\right)-\left(f_{b1}/F_{bE}\right)^2\right]} \le 1.0 \quad (3.9\text{-}3)$$

and

$$\frac{f_c}{F_{cE2}} + \left(\frac{f_{b1}}{F_{bE}}\right)^2 < 1.0 \quad (3.9\text{-}4)$$



where:

$f_c < F_{cE1} = \dfrac{0.822\,E_{min}'}{(\ell_{e1}/d_1)^2}$ for either uniaxial edgewise bending or biaxial bending

and

$f_c < F_{cE2} = \dfrac{0.822\,E_{min}'}{(\ell_{e2}/d_2)^2}$ for uniaxial flatwise bending or biaxial bending

and

$f_{b1} < F_{bE} = \dfrac{1.20\,E_{min}'}{(R_B)^2}$ for biaxial bending

f_{b1} = actual edgewise bending stress (bending load applied to narrow face of member), psi

f_{b2} = actual flatwise bending stress (bending load applied to wide face of member), psi

d_1 = wide face dimension (see Figure 3H), in.

d_2 = narrow face dimension (see Figure 3H), in.

Example

top chord 2x10

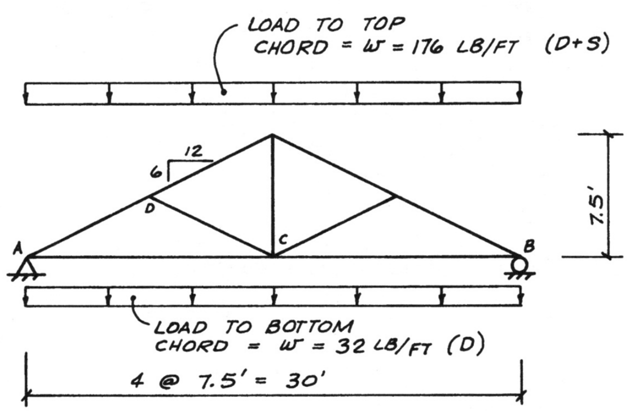
4. Calculate the **actual** axial and flexural stress.

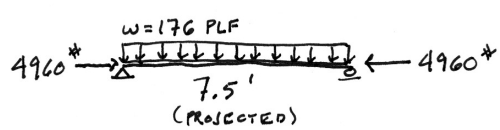
2x10:

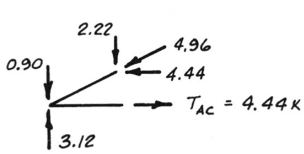
A = 13.875 in^2 f_c = 357.5 psi

Sx = 21.39 in^3 f_{b1} = 694.2 psi

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]}$$







$$f_c = \frac{P}{A} = \frac{4960\#}{1.5 \times 9.25} = 357.5 \text{ psi}$$

$$f_b = \frac{M}{S_x} = \frac{1237.5\,(12)}{21.39} = 694.2 \text{ psi}$$

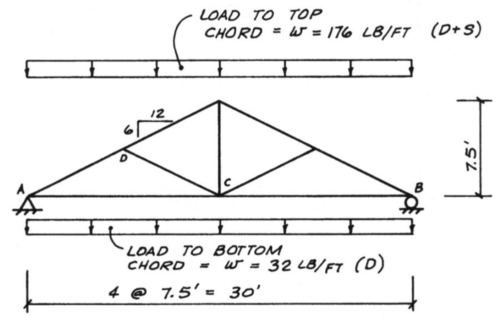
$$M = \frac{w\ell^2}{8} = \frac{176 \text{ PLF}(7.5')^2}{8} = 1237.5\ '\text{-}\#$$

$$S_x = 21.39 \text{ in}^3$$

Example

top chord 2x10

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]}$$

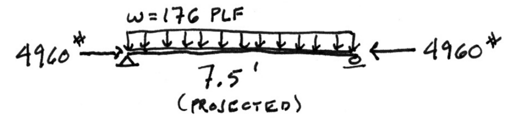


5. Determine **allowable** stresses using applicable factors:

   (compression: D+S)
   $F_c' = F_c\,(C_D\ C_F\ C_P)$
   $F_c' = 1350\ (1.15\ \ 1.0\ \ 0.897\ ) = 1392.6$ psi > 357.5

   (flexure: D+S)
   $F_b' = F_b\,(C_D\ C_L\ C_F)$
   $F_b' = 1100\ (1.15\ \ 1.0\ \ 1.1) = 1391.5$ psi > 694.2

$$C_P = \frac{1+\left(F_{cE}/F_c^*\right)}{2c} - \sqrt{\left[\frac{1+\left(F_{cE}/F_c^*\right)}{2c}\right]^2 - \frac{F_{cE}/F_c^*}{c}} \quad (3.7\text{-}1)$$



C_P

$l_e = 8.385'$ $\quad d = 9.25''$

$l_e/d = \frac{8.385(12)}{9.25} = 10.88$

$F_{cE} = \frac{0.822\,E_{min}'}{(l_e/d)^2} = \frac{0.822\,(550000)}{10.88^2} = 3820$ psi

$F_c^* = 1350\,(1.15\ \ 1.0) = 1552.5$ psi

$F_{cE}/F_c^* = \frac{3820}{1552} = 2.46 \qquad c = 0.8$

$C_P = 0.897$

Example

top chord 2x10

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]}$$

Size Factors, C_F

		F_b		F_t	F_c
		Thickness (breadth)			
Grades	Width (depth	2" & 3"	4"		
Select Structural, No.1 & Btr, No.1, No.2, No.3	2", 3", & 4"	1.5	1.5	1.5	1.15
	5"	1.4	1.4	1.4	1.1
	6"	1.3	1.3	1.3	1.1
	8"	1.2	1.3	1.2	1.05
	10"	1.1	1.2	1.1	1.0
	12"	1.0	1.1	1.0	1.0
	14" & wider	0.9	1.0	0.9	0.9
Stud	2", 3", & 4"	1.1	1.1	1.1	1.05
	5" & 6"	1.0	1.0	1.0	1.0
	8" & wider	Use No.3			
Construction Standard	2", 3", & 4"	1.0	1.0	1.0	1.0
Utility	4"	1.0	1.0	1.0	1.0
	2" & 3"	0.4	—	0.4	0.6

5. Determine **allowable** stresses using applicable factors:

   (compression: D+S)
   $F_c' = F_c\,(C_D\ C_F\ C_P)$
   $F_c' = 1350\ (1.15\ \ 1.0\ \ 0.897\ ) = 1392.6$ psi > 357.5

   (flexure: D+S)
   $F_b' = F_b\,(C_D\ C_L\ C_F)$
   $F_b' = 1100\ (1.15\ \ 1.0\ \ 1.1) = 1391.5$ psi > 694.2

3.3.3 Beam Stability Factor, C_L

3.3.3.3 When the compression edge of a bending member is supported throughout its length to prevent lateral displacement, and the ends at points of bearing have lateral support to prevent rotation, $C_L = 1.0$.

Example

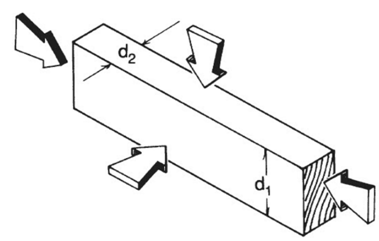
top chord 2x10

Eq. 3.9-3

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-(f_c/F_{cE1})\right]} \le 1.0$$

COMP. + FLEXURE X-X

Figure 3H Combined Bending and Axial Compression



where:

EULER 1

$f_c < F_{cE1} = \dfrac{0.822\,E_{min}'}{(\ell_{e1}/d_1)^2}$ for either uniaxial edge-wise bending or biaxial bending

and

EULER 2

$f_c < F_{cE2} = \dfrac{0.822\,E_{min}'}{(\ell_{e2}/d_2)^2}$ for uniaxial flatwise bending or biaxial bending

and

LTB

$f_{b1} < F_{bE} = \dfrac{1.20\,E_{min}'}{(R_B)^2}$ for biaxial bending

f_{b1} = actual edgewise bending stress (bending load applied to narrow face of member)

f_{b2} = actual flatwise bending stress (bending load applied to wide face of member)

d_1 = wide face dimension (see Figure 3H)

d_2 = narrow face dimension (see Figure 3H)

COMPRESSION:

$$\left[\frac{f_c}{F_c'}\right]^2 = \left[\frac{357.5}{1392.6}\right]^2 = 0.0659$$

Eq. 3.9-3

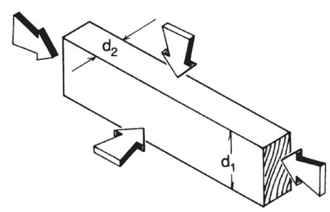
Example

top chord 2x10

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-(f_c/F_{cE1})\right]} \le 1.0$$

COMP. + FLEXURE X-X

Figure 3H Combined Bending and Axial Compression



EULER 1

$f_c < F_{cE1} = \dfrac{0.822\,E_{min}'}{(\ell_{e1}/d_1)^2}$ for either uniaxial edge-wise bending or biaxial bending

f_{b1} = actual edgewise bending stress (bending load applied to narrow face of member)

d_1 = wide face dimension (see Figure 3H)

d_2 = narrow face dimension (see Figure 3H)

$f_c = \dfrac{P}{A} = \dfrac{4960^*}{1.5 \times 9.25} = 357.5$ psi

$f_b = \dfrac{M}{S_x} = \dfrac{1237.5\,(12)}{21.39} = 694.2$ psi

$M = \dfrac{w\ell^2}{8} = \dfrac{176\text{ PLF}(7.5')^2}{8} = 1237.5\ '\text{-}\#$

$S_x = 21.39\ \text{in}^3$

$f_c = 357.5$ psi

$E_{min} = 550000$ psi

$\ell_{e1} = 8.385'$

$d_1 = 9.25''$

$\ell_{e1}/d_1 = \dfrac{8.385(12)}{9.25''} = 10.88$

$F_{CE1} = \dfrac{0.822\,(550000)}{10.88^2}$

$= 3820$ psi

FLEXURE:

$$\frac{f_{b1}}{F_{b1}'} = \frac{694.2}{1392} = 0.4987$$

AMPLIFICATION FACTOR:

$$\frac{1}{1-(357.5/3820)} = \frac{1}{0.906}$$

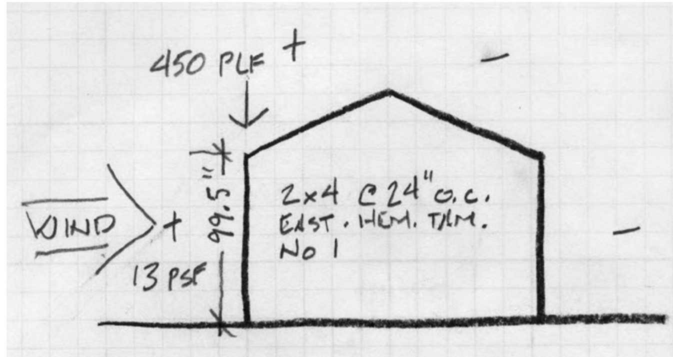
$0.4987\,(1.103) = 0.550$

COMBINATION:

$0.0659 + 0.550 = 0.616$

$0.616 < 1.0$ ✓ PASS

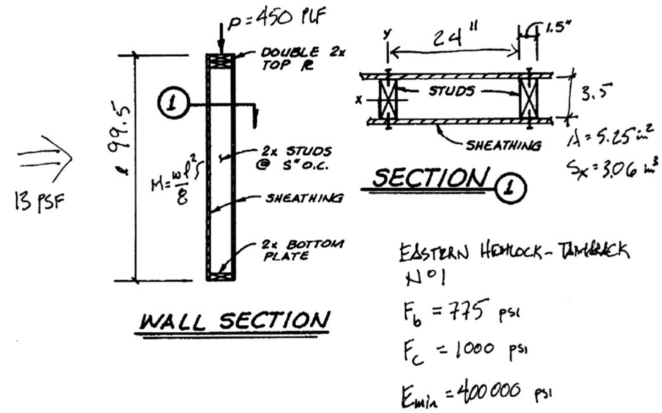
Combined Stress in NDS
procedure



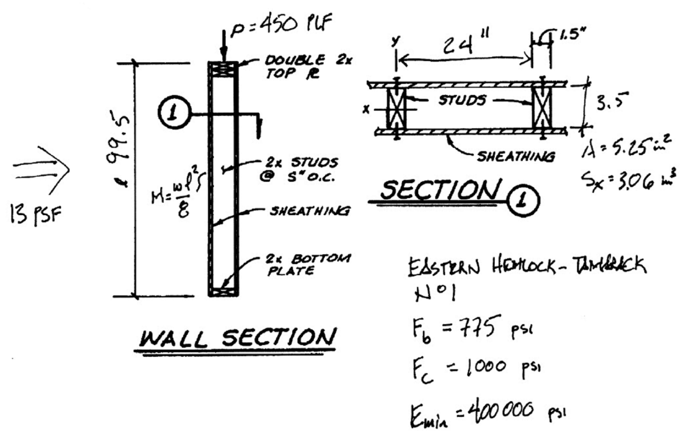
Exterior stud wall under
bending + axial compression

1. Determine load per stud
2. Use axial load and moment to find actual stresses f_c and f_b
3. Determine load factors
4. Calculate factored stresses
5. Check NDS equations

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]} \leq 1.0 \quad (3.9\text{-}3)$$



Combined Stress in NDS
example

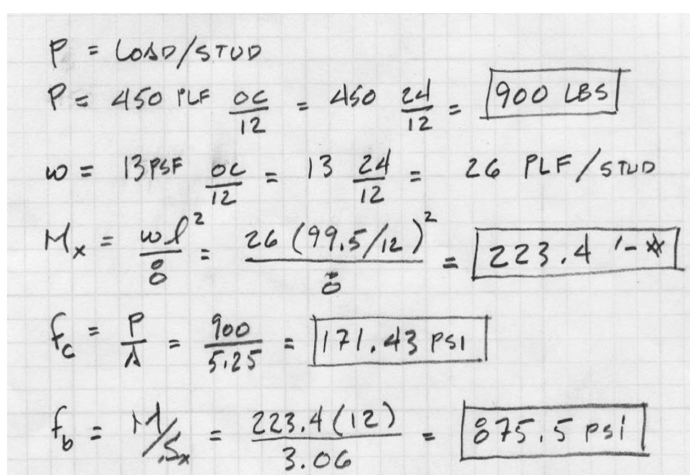


Exterior stud wall under
bending + axial compression

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]} \leq 1.0 \quad (3.9\text{-}3)$$

1. Determine load per stud

2. Use axial load and moment to find actual stresses fc and fb



P = LOAD/STUD

$$P = 450 \text{ PLF } \frac{oc}{12} = 450\,\frac{24}{12} = \boxed{900 \text{ LBS}}$$

$$w = 13 \text{PSF } \frac{oc}{12} = 13\,\frac{24}{12} = 26 \text{ PLF/STUD}$$

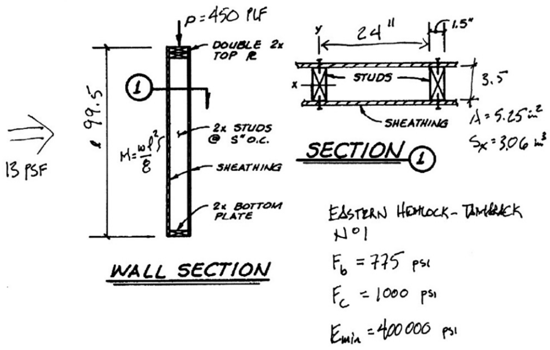
$$M_x = \frac{w\ell^2}{8} = \frac{26\,(99.5/12)^2}{8} = \boxed{223.4 \text{ '-\#}}$$

$$f_c = \frac{P}{A} = \frac{900}{5.25} = \boxed{171.43 \text{ PSI}}$$

$$f_b = M/S_x = \frac{223.4\,(12)}{3.06} = \boxed{875.5 \text{ psi}}$$

Combined Stress in NDS
example

Exterior stud wall under
bending + axial compression



3. Determine load factors (bending)

Size Factors, C_F

		F_b		F_t	F_c
		Thickness (breadth)			
Grades	Width (depth)	2" & 3"	4"		
Select Structural, No.1 & Btr, No.1, No.2, No.3	2", 3", & 4"	1.5	1.5	1.5	1.15
	5"	1.4	1.4	1.4	1.1
	6"	1.3	1.3	1.3	1.1
	8"	1.2	1.3	1.2	1.05
	10"	1.1	1.2	1.1	1.0
	12"	1.0	1.1	1.0	1.0
	14" & wider	0.9	1.0	0.9	0.9
Stud	2", 3", & 4"	1.1	1.1	1.1	1.05
	5" & 6"	1.0	1.0	1.0	1.0
	8" & wider	Use No.3			
Construction Standard	2", 3", & 4"	1.0	1.0	1.0	1.0
Utility	4"	1.0	1.0	1.0	1.0
	2" & 3"	0.4	—	0.4	0.6

$F_b = 775$ psi $\quad F_c = 1000$ psi $\quad E_{min} = 400\,000$ psi

FACTORS :

$C_D = \boxed{1.6}$ (WIND)

$C_F = \boxed{1.5}$ (FOR F_b) $\quad \boxed{1.15}$ (FOR F_c)

$C_L = \boxed{1.0}$ (BRACED BY SHEATHING)

$C_r = \boxed{1.15}$ ($\leq 24''$ o.c.)

Combined Stress in NDS
example

Exterior stud wall under
bending + axial compression

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]}$$

$F_b = 775$ psi

$C_D = 1.6 \qquad C_F = 1.5$

$C_M = 1.0 \qquad C_{fu} = 1.0$

$C_t = 1.0 \qquad C_i = 1.0$

$C_L = 1.0 \qquad C_r = 1.15$

4. Calculate factored stresses
(bending stress)

$$F_b' = 775(1.6)(1.5)(1.15)$$
$$= 2139 \text{ psi}$$

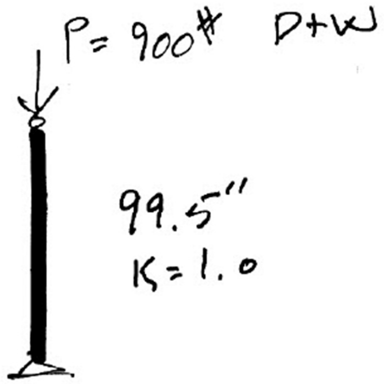
Combined Stress in NDS
example

$$C_P = \frac{1+\left(F_{cE}/F_c^*\right)}{2c} - \sqrt{\left[\frac{1+\left(F_{cE}/F_c^*\right)}{2c}\right]^2 - \frac{F_{cE}/F_c^*}{c}}$$

Exterior stud wall under
bending + axial compression

$$F_{cE} = \frac{0.822\ E_{min}'}{\left(\ell_e/d\right)^2}$$

c = 0.8 for sawn lumber



C_p

$F^* = 1000(1.6)(1.15) = 1840$

$F_{CE} = \frac{0.822(400000)}{(99.5/3.5)^2} = 406.8$

$c = 0.8$

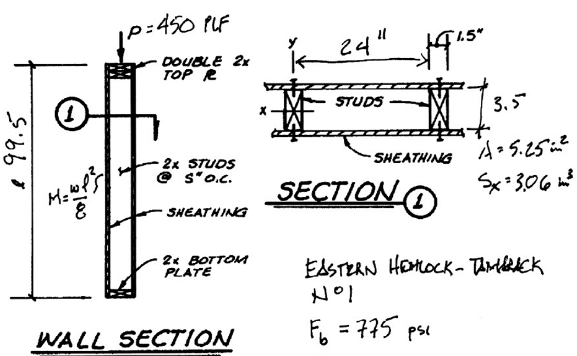
$C_p = 0.21$

3. Determine load factors (compression)

Combined Stress in NDS
example

Exterior stud wall under
bending + axial compression

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-\left(f_c/F_{cE1}\right)\right]}$$



EASTERN HEMLOCK-TAMARACK
N°1
$F_b = 775$ psi
$F_c = 1000$ psi
$E_{min} = 400000$ psi

4. Calculate stresses (compression stress)

Actual Stress

$f_c = \frac{P}{A} = \frac{900^{\#}}{5.25} = 171.4$ psi

Factored Allowable Stress

$F_c' = 1000(1.6)(1.15)(0.21) = 386.4$ psi

Combined Stress in NDS
example

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'\left[1-(f_c/F_{cE1})\right]} \le 1.0$$

COMP. + FLEXURE X-X

Exterior stud wall under
bending + axial compression

$$f_c = \frac{P}{A} = \frac{900}{5.25} = \boxed{171.43 \text{ psi}}$$

$$f_b = M/S_x = \frac{223.4(12)}{3.06} = \boxed{875.5 \text{ psi}}$$

5. Combined Stress Calculation (eq. 3.9-3)

$$F_{cE} = \frac{0.822\ E_{min}'}{(\ell_e/d)^2}$$

$$F_{cE} = \frac{0.822(400000)}{(99.5/3.5)^2} = 406.8$$

$$\left[\frac{f_c}{F_c'}\right]^2 + \frac{f_{b1}}{F_{b1}'}\ \frac{1}{1-(f_c/F_{cE1})} \le 1.0$$

$$\left[\frac{171.4}{386.4}\right]^2 + \frac{876}{2139}\ \frac{1}{1-(171.4/406.8)}$$

$$0.1967 + (0.4095)(1.728) =$$

$$0.1967 + 0.7077 = 0.9045 \le 1.0 \ \checkmark \text{ok}$$