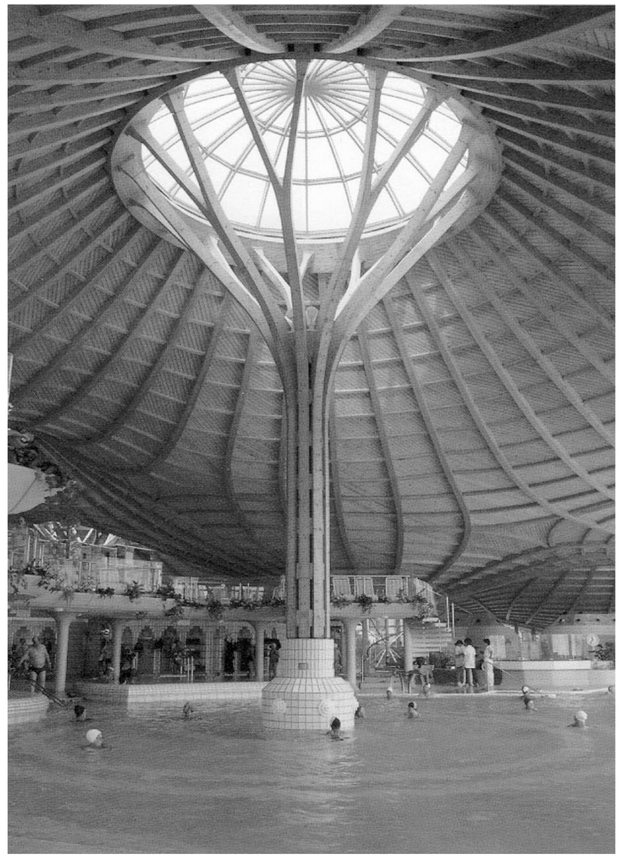
Architecture 324
Structures II

Wood Column Analysis

- Failure Modes
- Euler Equation
- End Conditions and Lateral Bracing
- Analysis of Wood Columns



Solemar, Bad Dürrheim
Klaus Linkwitz, 1987

Failure Modes

	FLEXURE	AXIAL
Strength	$f_b = \frac{Mc}{I}$ $f_v = \frac{VQ}{Ib}$	$f_c = \frac{P}{A}$
Stability	$C_L = \frac{1+(F_{bE}/F_b^*)}{1.9} - \sqrt{\left[\frac{1+(F_{bE}/F_b^*)}{1.9}\right]^2 - \frac{F_{bE}/F_b^*}{0.95}}$	$C_P = \frac{1+(F_{cE}/F_c^*)}{2c} - \sqrt{\left[\frac{1+(F_{cE}/F_c^*)}{2c}\right]^2 - \frac{F_{cE}/F_c^*}{c}}$
Serviceability	**Deflection (or Bearing)**	**Bearing (crushing limit)**

Leonhard Euler (1707 – 1783)

Euler Buckling (elastic buckling)

$$P_{cr} = \frac{\pi^2 AE}{\left(\frac{KL}{r}\right)^2} = \frac{\pi^2 IE}{(KL)^2}$$

$$r = \sqrt{\frac{I}{A}}$$

$$I = Ar^2$$

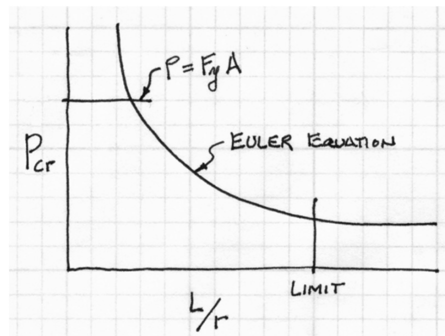
portrait by Emanuel Handmann,1753

- A = Cross sectional area (in^2)
- E = Modulus of elasticity of the material (lb/in^2)
- K = Stiffness (curvature mode) factor
- L = Column length between pinned ends (in.)
- r = radius of gyration (in.)

$$f_{cr} = \frac{\pi^2 E}{\left(\frac{KL}{r}\right)^2}$$

$$F_{cE} = \frac{0.822\ E'_{min}}{\left(\frac{le}{d}\right)^2}$$

$$r = d/\sqrt{12}$$



Failure Mode - Strength

Short Columns – fail by crushing

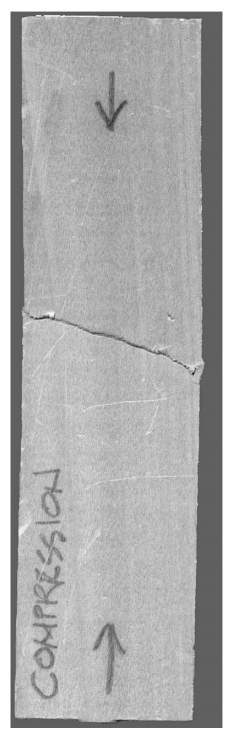
Analysis

$$f_c = \frac{P}{A} \leq F'_c$$

Design

$$A = \frac{P}{F'_c}$$

- f_c = Actual compressive stress
- A = Cross-sectional area of column (in^2)
- P = Load on the column
- F'_c = Allowable compressive stress per codes



Failure Modes – Stability

Long Columns – fail by buckling

Traditional Euler

$$f_{cr} = \frac{\pi^2 E}{\left(\frac{KL}{r}\right)^2}$$

- E = Modulus of elasticity of the column material (psi)
- K = Stiffness (curvature mode) factor
- L = Column length between ends (inches)
- r = radius of gyration = $\sqrt{I/A}$ (inches)

$$r = d/\sqrt{12}$$

NDS Equation

$$F_{cE} = \frac{0.822E'_{min}}{\left(\frac{l_e}{d}\right)^2}$$

- E'min = reduced E modulus (psi)
- le = Ke l_u (inches)
- d (inches)
- 0.822 = $\pi^2/12$

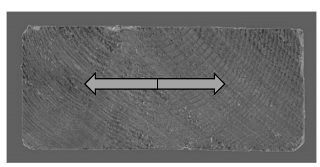
Slenderness Ratio le/d

Slenderness Ratios:

The larger ratio will fail first.
Try to balance for efficiency.

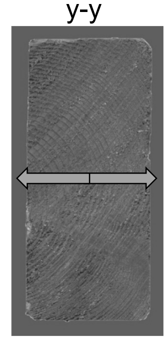
Slenderness Limited to < 50

x-x



d = 3.5

y-y



b = 1.5

X- X

$K_e = 1.0$

$l_e = 1.0(96)$

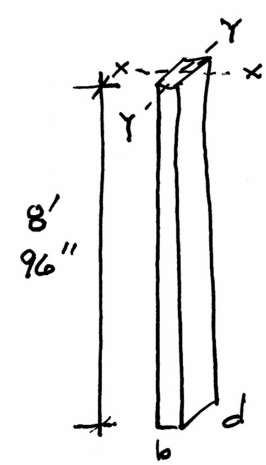
$\frac{l_e}{d} = \frac{96}{3.5} = 27.4$

Y- Y

$K_e = 1.0$

$l_e = 1.0(96)$

$\frac{l_e}{b} = \frac{96}{1.5} = 64$



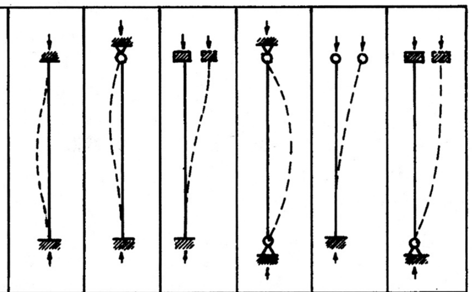
End Support Conditions, K_e

K_e is a constant based on the end conditions
ℓ is the actual length
ℓ_e is the effective length (curved part)

$\ell_e = K_e \ell$

use these →

Table G1 Buckling Length Coefficients, K_e

Buckling modes						
Theoretical K_e value	0.5	0.7	1.0	1.0	2.0	2.0
Recommended design K_e when ideal conditions approximated	0.65	0.80	1.2	1.0	2.10	2.4
End condition code	Rotation fixed, translation fixed Rotation free, translation fixed Rotation fixed, translation free Rotation free, translation free					

Allowable Flexure Stress F_c'

F_c from tables determined by species and grade

$F_c' = F_c$ (adjustment factors)

Actual Flexure Stress f_b

$f_c = P/A$

$$F_c' \geq f_c$$

Table 4A (Cont.) **Reference Design Values for Visually Graded Dimension Lumber (2" - 4" thick)[1,2,3]**

(All species except Southern Pine—see Table 4B) (Tabulated design values are for normal load duration and dry service conditions. See NDS 4.3 for a comprehensive description of design value adjustment factors.)

USE WITH TABLE 4A ADJUSTMENT FACTORS

Species and commercial grade	Size classification	Design values in pounds per square inch (psi) Bending F_b	Tension parallel to grain F_t	Shear parallel to grain F_v	Compression perpendicular to grain $F_{c\perp}$	Compression parallel to grain F_c	Modulus of Elasticity E	E_{min}	Specific Gravity[4] G	Grading Rules Agency
SPRUCE-PINE-FIR										
Select Structural	2" & wider	1,250	700	135	425	1,400	1,500,000	550,000	0.42	NLGA
No. 1/ No. 2		875	450	135	425	1,150	1,400,000	510,000		
No. 3		500	250	135	425	650	1,200,000	440,000		
Stud	2" & wider	675	350	135	425	725	1,200,000	440,000		
Construction	2" - 4" wide	1,000	500	135	425	1,400	1,300,000	470,000		
Standard		550	275	135	425	1,150	1,200,000	440,000		
Utility		275	125	135	425	750	1,100,000	400,000		

Adjustment Factors

Table 4.3.1 Applicability of Adjustment Factors for Sawn Lumber

		ASD only	ASD and LRFD											LRFD only		
		Load Duration Factor	Wet Service Factor	Temperature Factor	Beam Stability Factor	Size Factor	Flat Use Factor	Incising Factor	Repetitive Member Factor	Column Stability Factor	Buckling Stiffness Factor	Bearing Area Factor	Format Conversion Factor K_F	Resistance Factor ϕ	Time Effect Factor
$F_b' = F_b$	x	C_D	C_M	C_t	C_L	C_F	C_{fu}	C_i	C_r	-	-	-	2.54	0.85	λ
$F_t' = F_t$	x	C_D	C_M	C_t	-	C_F	-	C_i	-	-	-	-	2.70	0.80	λ
$F_v' = F_v$	x	C_D	C_M	C_t	-	-	-	C_i	-	-	-	-	2.88	0.75	λ
$F_c' = F_c$	x	C_D	C_M	C_t	-	C_F	-	C_i	-	C_P	-	-	2.40	0.90	λ
$F_{c\perp}' = F_{c\perp}$	x	-	C_M	C_t	-	-	-	C_i	-	-	-	C_b	1.67	0.90	-
$E' = E$	x	-	C_M	C_t	-	-	-	C_i	-	-	-	-	-	-	-
$E_{min}' = E_{min}$	x	-	C_M	C_t	-	-	-	C_i	-	-	C_T	-	1.76	0.85	-

Allowable Flexure Stress F_c'

F_c from tables determined by species and grade

$F_c' = F_c (C_D\ C_M\ C_t\ C_F\ C_i\ C_P)$

Adjustment factors for compression:
- C_D Load Duration Factor
- C_t Temperature Factor

Table 2.3.2 Frequently Used Load Duration Factors, C_D[1]

Load Duration	C_D	Typical Design Loads
Permanent	0.9	Dead Load
Ten years	1.0	Occupancy Live Load
Two months	1.15	Snow Load
Seven days	1.25	Construction Load
Ten minutes	1.6	Wind/Earthquake Load
Impact[2]	2.0	Impact Load

(1) Actual stress due to (DL) ≤ (0.9) (Design value)
(2) Actual stress due to (DL+LL) ≤ (1.0) (Design value)
(3) Actual stress due to (DL+WL) ≤ (1.6) (Design value)
(4) Actual stress due to (DL+LL+SL) ≤ (1.15) (Design value)
(5) Actual stress due to (DL+LL+WL) ≤ (1.6) (Design value)
(6) Actual stress due to (DL+SL+WL) ≤ (1.6) (Design value)
(7) Actual stress due to (DL+LL+SL+WL) ≤ (1.6) (Design value)

Table 2.3.3 Temperature Factor, C_t

Reference Design Values	In-Service Moisture Conditions[1]	C_t		
		T≤100°F	100°F<T≤125°F	125°F<T≤150°F
F_t, E, E_{min}	Wet or Dry	1.0	0.9	0.9
F_b, F_v, F_c, and $F_{c\perp}$	Dry	1.0	0.8	0.7
	Wet	1.0	0.7	0.5

1. Wet and dry service conditions for sawn lumber, structural glued laminated timber, prefabricated wood I-joists, structural composite lumber, wood structural panels and cross-laminated timber are specified in 4.1.4, 5.1.4, 7.1.4, 8.1.4, 9.3.3, and 10.1.5 respectively.

Allowable Flexure Stress F_c'
(For **Dimensioned Lumber**)

F_c from tables determined by species and grade

$F_c' = F_c (C_D C_M C_t C_F C_i C_P)$

Adjustment factors for compression:
- C_M Moisture Factor
- C_F Size Factor

Wet Service Factor, C_M

When dimension lumber is used where moisture content will exceed 19% for an extended time period, design values shall be multiplied by the appropriate wet service factors from the following table:

Wet Service Factors, C_M

F_b	F_t	F_v	$F_{c\perp}$	F_c	E and E_{min}
0.85*	1.0	0.97	0.67	0.8**	0.9

* when $(F_b)(C_F) \le 1{,}150$ psi, $C_M = 1.0$

** when $(F_c)(C_F) \le 750$ psi, $C_M = 1.0$

Size Factors, C_F

Grades	Width (depth)	F_b Thickness (breadth) 2" & 3"	F_b Thickness (breadth) 4"	F_t	F_c
Select Structural, No.1 & Btr, No.1, No.2, No.3	2", 3", & 4"	1.5	1.5	1.5	1.15
	5"	1.4	1.4	1.4	1.1
	6"	1.3	1.3	1.3	1.1
	8"	1.2	1.3	1.2	1.05
	10"	1.1	1.2	1.1	1.0
	12"	1.0	1.1	1.0	1.0
	14" & wider	0.9	1.0	0.9	0.9
Stud	2", 3", & 4"	1.1	1.1	1.1	1.05
	5" & 6"	1.0	1.0	1.0	1.0
	8" & wider	Use No.3 Grade tabulated design values and size factors			
Construction, Standard	2", 3", & 4"	1.0	1.0	1.0	1.0
Utility	4"	1.0	1.0	1.0	1.0
	2" & 3"	0.4	—	0.4	0.6

Allowable Flexure Stress F_c'
(For **Timbers**)

F_c from tables determined by species and grade

$F_c' = F_c (C_D C_M C_t C_F C_i C_P)$

Adjustment factors for compression:
- C_M Moisture Factor
- C_F Size Factor

Size Factor, C_F

When visually graded timbers are subjected to loads applied to the narrow face, tabulated design values shall be multiplied by the following size factors:

Size Factors, C_F

Depth	F_b	F_t	F_c
d > 12"	$(12/d)^{1/9}$	1.0	1.0
d ≤ 12"	1.0	1.0	1.0

Wet Service Factor, C_M

When timbers are used where moisture content will exceed 19% for an extended time period, design values shall be multiplied by the appropriate wet service factors from the following table (for Southern Pine and Mixed Southern Pine, use tabulated design values without further adjustment):

Wet Service Factors, C_M

F_b	F_t	F_v	$F_{c\perp}$	F_c	E and E_{min}
1.00	1.00	1.00	0.67	0.91	1.00

Allowable Flexure Stress F_c'

F_c from tables determined by species and grade

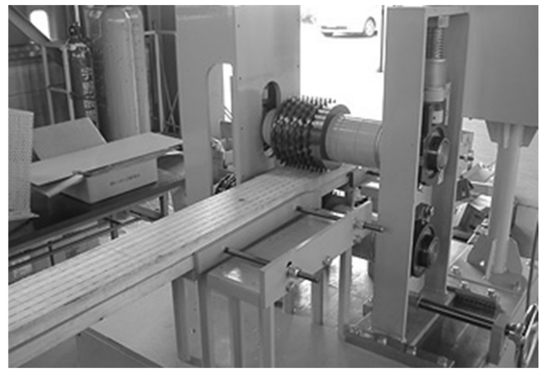
$F_c' = F_c (C_D\ C_M\ C_t\ C_F\ C_i\ C_P)$

Adjustment factors for compression :

C_i Incising Factor

Table 4.3.8 Incising Factors, C_i

Design Value	C_i
E, E_{min}	0.95
F_b, F_t, F_c, F_v	0.80
$F_{c\perp}$	1.00

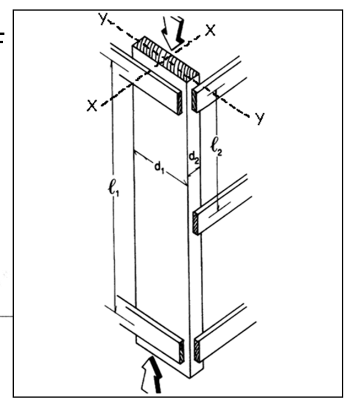


Allowable Compression Stress F_c'

F_c from tables determined by species and grade

$F_c' = F_c (C_D\ C_M\ C_t\ C_F\ C_i\ C_P)$

Fig. 3F



3.7 Solid Columns

3.7.1 Column Stability Factor, C_P

3.7.1.1 When a compression member is supported throughout its length to prevent lateral displacement in all directions, $C_P = 1.0$.

3.7.1.2 The effective column length, ℓ_e, for a solid column shall be determined in accordance with principles of engineering mechanics. One method for determining effective column length, when end-fixity conditions are known, is to multiply actual column length by the appropriate effective length factor specified in Appendix G, $\ell_e = (K_e)(\ell)$.

3.7.1.3 For solid columns with rectangular cross section, the slenderness ratio, ℓ_e/d, shall be taken as the larger of the ratios ℓ_{e1}/d_1 or ℓ_{e2}/d_2 (see Figure 3F) where each ratio has been adjusted by the appropriate buckling length coefficient, K_e, from Appendix G.

3.7.1.4 The slenderness ratio for solid columns, ℓ_e/d, shall not exceed 50, except that during construction ℓ_e/d shall not exceed 75.

3.7.1.5 The column stability factor shall be calculated as follows:

$$C_P = \frac{1+(F_{cE}/F_c^*)}{2c} - \sqrt{\left[\frac{1+(F_{cE}/F_c^*)}{2c}\right]^2 - \frac{F_{cE}/F_c^*}{c}} \quad (3.7\text{-}1)$$

where:

F_c^* = reference compression design value parallel to grain multiplied by all applicable adjustment factors except C_P (see 2.3), psi

$$F_{cE} = \frac{0.822\ E_{min}'}{(\ell_e/d)^2}$$

c = 0.8 for sawn lumber

c = 0.85 for round timber poles and piles

c = 0.9 for structural glued laminated timber or structural composite lumber

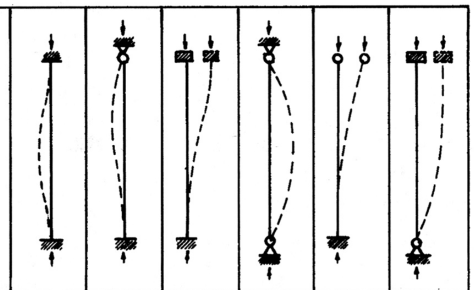
End Support Conditions, K_e

K_e is a constant based on the end conditions
ℓ is the actual length
ℓ_e is the effective length (curved part)

$\ell_e = K_e \ell$

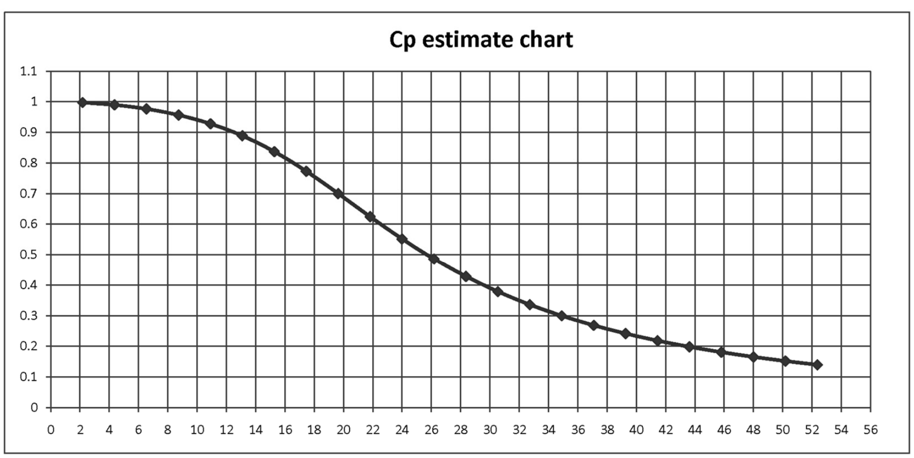
use these →

Table G1 Buckling Length Coefficients, K_e

Buckling modes						
Theoretical K_e value	0.5	0.7	1.0	1.0	2.0	2.0
Recommended design K_e when ideal conditions approximated	0.65	0.80	1.2	1.0	2.10	2.4
End condition code	Rotation fixed, translation fixed Rotation free, translation fixed Rotation fixed, translation free Rotation free, translation free					

C_P

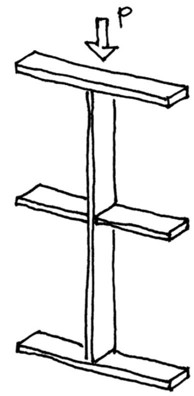
C_P



ℓ / d

This is an estimate – not the exact value

Analysis of Wood Columns



Data:

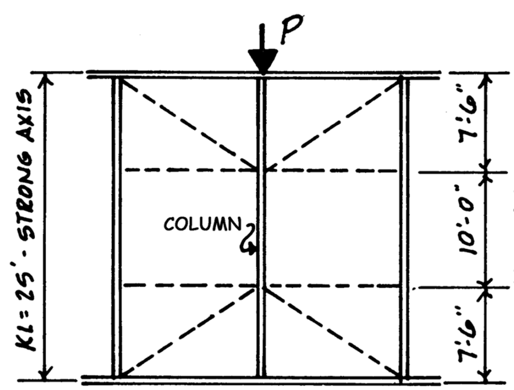
- Column – <u>size</u>, length
- Support conditions
- Material properties – F_c , E_{min}
- Load

Required:

- Pass/Fail or margin of safety

1. Calculate slenderness ratio l_e/d
   largest ratio governs. Must be < 50
2. Find adjustment factors
   C_D C_M C_t C_F C_i
3. Calculate C_P
4. Determine allowable F'_c by multiplying the tabulated F_c by all the above factors
5. Calculate the actual stress: $f_c = P/A$
6. Compare Allowable and Actual stress.
   $F'_c > f_c$ passes

Analysis Example: Pass/Fail



WALL ELEVATION

Data: section 4x8 (nominal)
Douglas Fir-Larch No1
M.C. 15%
P = 7000 LBS (Snow Load)

Find: Pass/Fail

From NDS Supplement Table 4A

F_c = 1500 psi

E_{min} = 620000 psi

C_D = 1.15 (snow)

C_M = 1.0

C_t = 1.0

C_F = 1.05 (4x8)

C_i = 1.0

C_P = ?

Species and commercial grade	Size classification	Compression perpendicular to grain $F_{c\perp}$	Compression parallel to grain F_c	Modulus of Elasticity E	E_{min}
DOUGLAS FIR-LARCH					
Select Structural		625	1,700	1,900,000	690,000
No. 1 & Btr		625	1,550	1,800,000	660,000
No. 1	2" & wider	625	1,500	1,700,000	620,000
No. 2		625	1,350	1,600,000	580,000
No. 3		625	775	1,400,000	510,000
Stud	2" & wider	625	850	1,400,000	510,000
Construction		625	1,650	1,500,000	550,000
Standard	2" - 4" wide	625	1,400	1,400,000	510,000
Utility		625	900	1,300,000	470,000

Analysis Example: Pass/Fail

Determine Allowable Flexure Stress F_c'

$F_c = 1500$ psi

$F_c' = F_c\,(C_D\,C_M\,C_t\,C_F\,C_i\,C_P)$

$E'_{min} = E_{min}\,(C_m\,C_t\,C_i\,C_T)$

Adjustment factors for compression:

- C_M Moisture Factor = 1.0 (dry)
- C_F Size Factor = 1.05

Wet Service Factor, C_M

When dimension lumber is used where moisture content will exceed 19% for an extended time period, design values shall be multiplied by the appropriate wet service factors from the following table:

Wet Service Factors, C_M

F_b	F_t	F_v	$F_{c\perp}$	F_c	E and E_{min}
0.85*	1.0	0.97	0.67	0.8**	0.9

* when $(F_b)(C_F) \le 1{,}150$ psi, $C_M = 1.0$

** when $(F_c)(C_F) \le 750$ psi, $C_M = 1.0$

Size Factors, C_F

Grades	Width (depth)	F_b Thickness (breadth) 2" & 3"	F_b Thickness (breadth) 4"	F_t	F_c
Select Structural, No.1 & Btr, No.1, No.2, No.3	2", 3", & 4"	1.5	1.5	1.5	1.15
	5"	1.4	1.4	1.4	1.1
	6"	1.3	1.3	1.3	1.1
	8"	1.2	1.3	1.2	1.05
	10"	1.1	1.2	1.1	1.0
	12"	1.0	1.1	1.0	1.0
	14" & wider	0.9	1.0	0.9	0.9
Stud	2", 3", & 4"	1.1	1.1	1.1	1.05
	5" & 6"	1.0	1.0	1.0	1.0
	8" & wider	Use No.3 Grade tabulated design values and size factors			
Construction, Standard	2", 3", & 4"	1.0	1.0	1.0	1.0
Utility	4"	1.0	1.0	1.0	1.0
	2" & 3"	0.4	—	0.4	0.6

Analysis Example: Pass/Fail

Calculate C_P

$$C_P = \frac{1+(F_{cE}/F_c^*)}{2c} - \sqrt{\left[\frac{1+(F_{cE}/F_c^*)}{2c}\right]^2 - \frac{F_{cE}/F_c^*}{c}} \quad (3.7\text{-}1)$$

where:

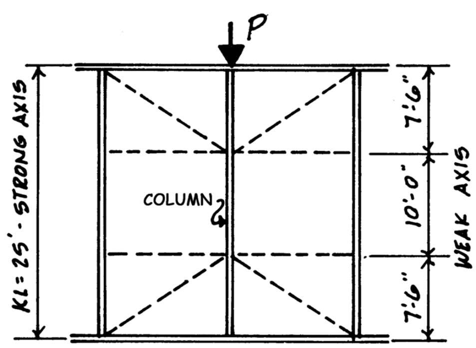
F_c^* = reference compression design value parallel to grain multiplied by all applicable adjustment factors except C_P (see 2.3), psi

$$F_{cE} = \frac{0.822\,E_{min}'}{(\ell_e/d)^2}$$

c = 0.8 for sawn lumber

c = 0.85 for round timber poles and piles

c = 0.9 for structural glued laminated timber or structural composite lumber



WALL ELEVATION

4x8 3.5"x7.25"

X-X

$l_{e_x} = 25' = 300"$

$l_{ex}/d_1 = \frac{300"}{7.25"} = 41.4$

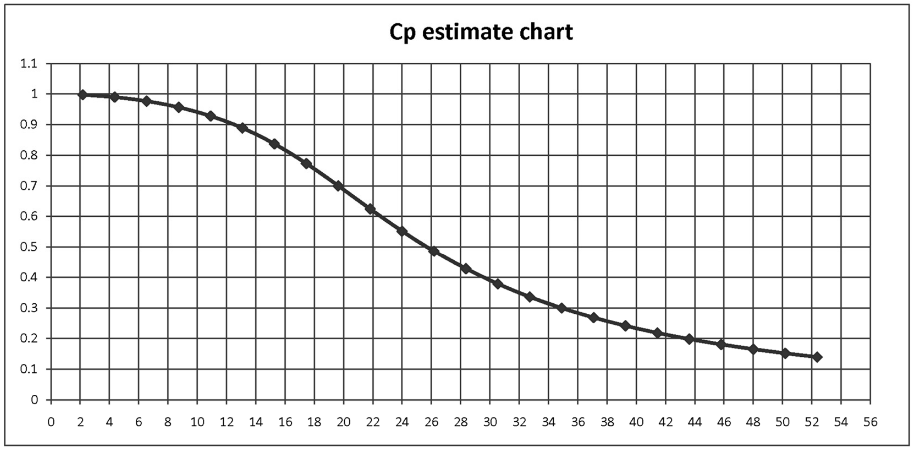
Y-Y

$l_{e_y} = 10' = 120"$

$l_{ey}/d_2 = \frac{120"}{3.5"} = 34.3$

$l_e/d = 41.4 < 50$ ✓

C_P



This is an estimate – not the exact value

Analysis Example: Pass/Fail

Calculate C_P

$$C_P = \frac{1+(F_{cE}/F_c^*)}{2c} - \sqrt{\left[\frac{1+(F_{cE}/F_c^*)}{2c}\right]^2 - \frac{F_{cE}/F_c^*}{c}} \quad (3.7\text{-}1)$$

where:

F_c^* = reference compression design value parallel to grain multiplied by all applicable adjustment factors except C_P (see 2.3), psi

$$F_{cE} = \frac{0.822\,E_{min}'}{(\ell_e/d)^2}$$

c = 0.8 for sawn lumber

c = 0.85 for round timber poles and piles

c = 0.9 for structural glued laminated timber or structural composite lumber

$E_{min}' = E_{min}$	x	C_M	C_t	C_i	C_T

$$F_{cE} = \frac{0.822\,E'_{min}}{(\ell_e/d)^2} = \frac{0.822(620000)}{(41.4)^2} = 297.6 \text{ psi}$$

$$F_c^* = 1500(1.15\ \ 1.05) = 1811.25 \text{ psi}$$

$$F_{cE}/F_c^* = \frac{297.6}{1811.25} = 0.164$$

$$c = 0.8$$

$$C_P = \frac{1+0.164}{2(0.8)} - \sqrt{\left[\frac{1+0.164}{2(0.8)}\right]^2 - \frac{0.164}{.8}}$$

$$C_P = 0.1584$$

Analysis Example: Pass/Fail

Determine Allowable stress

$$F'_c = 1500(1.15\ \ 1.05\ \ 0.1584)$$
$$= 286.9 \text{ psi}$$

Compare Allowable and Actual stress

$F'_c > f_c$ passes

$$f_c = \frac{P}{A} = \frac{7000\#}{25.38 \text{ in}^2} = 275.8 \text{ psi}$$

$F'_c > f_c$ ✓ OK

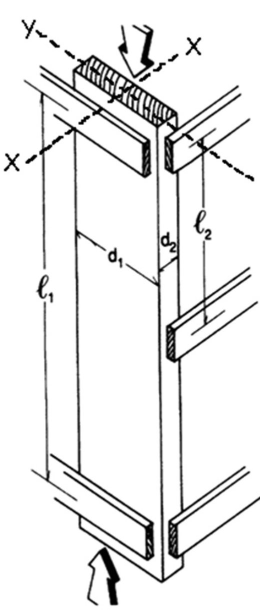
Capacity Analysis of Columns

Data:

- Column – <u>size</u>, length
- Support conditions
- Material properties – F_c , E_{min}

Required:

- Maximum Load Capacity, P_{max}



1. Calculate slenderness ratio l_e/d
   largest ratio governs. Must be < 50
2. Find adjustment factors
   C_D C_M C_t C_F C_i
3. Calculate C_P
4. Determine F'_c by multiplying the tabulated F_c by all the above factors
5. Set actual stress = allowable, $f_c = F'_c$
6. Find the maximum allowable load

   $P_{max} = F'_c\ A$

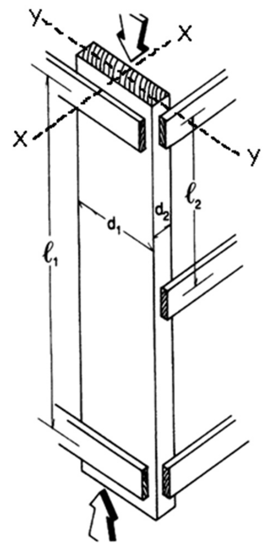
Analysis Example: Capacity

Data:

- 4x10
- Hem – Fir, No. 2   M.C. = 20%
- Wind Load
- $L_1 = 8'$ $L_2 = 4'$ $K_e=1.0$

Required:

- Maximum Load Capacity, Pmax



From NDS Supplement Table 4A

Fc = 1300 psi

Emin = 470000 psi

$C_D = 1.6$ (Table 2.3.2.)

$C_{Mc} = 0.8$ $C_{ME} = 0.9$

$C_t = 1.0$

$C_F = 1.0$ (chart in Supplement)

$C_i = 1.0$

$C_P = ?$

x - x

$\ell_{ex} = 8' = 96''$

$\frac{\ell_{ex}}{d_1} = \frac{96}{9.25} = 10.4$

y - y

$\ell_{ey} = 4' = 48''$

$\frac{\ell_{ey}}{d_2} = \frac{48}{3.5} = 13.7$

$\ell_e/d = 13.7 < 50$ ✓

Allowable Flexure Stress F_c'

4 x 10   M.C. = 20%

F_c from tables determined by species and grade

$F_c' = F_c (C_D\ C_M\ C_t\ C_F\ C_i\ C_P)$

Adjustment factors for compression:

C_M Moisture Factor $C_{Mc} = 0.8$ $C_{ME} = 0.9$

C_F Size Factor = 1.0

Wet Service Factor, C_M

When dimension lumber is used where moisture content will exceed 19% for an extended time period, design values shall be multiplied by the appropriate wet service factors from the following table:

Wet Service Factors, C_M

F_b	F_t	F_v	$F_{c\perp}$	F_c	E and E_{min}
0.85*	1.0	0.97	0.67	0.8**	0.9

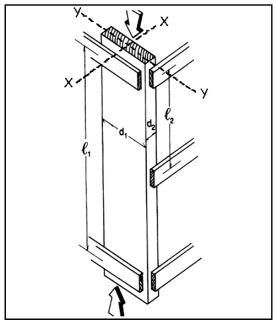
* when $(F_b)(C_F) \leq 1{,}150$ psi, $C_M = 1.0$

** when $(F_c)(C_F) \leq 750$ psi, $C_M = 1.0$

Size Factors, C_F

Grades	Width (depth)	F_b Thickness (breadth) 2" & 3"	F_b Thickness (breadth) 4"	F_t	F_c
Select Structural, No.1 & Btr, No.1, No.2, No.3	2", 3", & 4"	1.5	1.5	1.5	1.15
	5"	1.4	1.4	1.4	1.1
	6"	1.3	1.3	1.3	1.1
	8"	1.2	1.3	1.2	1.05
	10"	1.1	1.2	1.1	1.0
	12"	1.0	1.1	1.0	1.0
	14" & wider	0.9	1.0	0.9	0.9
Stud	2", 3", & 4"	1.1	1.1	1.1	1.05
	5" & 6"	1.0	1.0	1.0	1.0
	8" & wider	Use No.3 Grade tabulated design values and size factors			
Construction, Standard	2", 3", & 4"	1.0	1.0	1.0	1.0
Utility	4"	1.0	1.0	1.0	1.0
	2" & 3"	0.4	—	0.4	0.6

Capacity Example



Find C_P

$$F_{cE} = \frac{0.822\ E'_{min}}{(\ell_e/d)^2}$$

$$= \frac{0.822(470000\ (0.9))}{13.7^2}$$

$$= 1848.7\ psi$$

$$F_c^* = 1300(1.6\ \ 0.8)$$

$$= 1664.\ psi$$

$$F_{cE}/F_c^* = \frac{1848.7}{1664.} = 1.111$$

$$C_P = 0.7261$$

$$C_P = \frac{1+(F_{cE}/F_c^*)}{2c} - \sqrt{\left[\frac{1+(F_{cE}/F_c^*)}{2c}\right]^2 - \frac{F_{cE}/F_c^*}{c}} \quad (3.7\text{-}1)$$

where:

F_c^* = reference compression design value parallel to grain multiplied by all applicable adjustment factors except C_P (see 2.3), psi

$$F_{cE} = \frac{0.822\ E_{min}'}{(\ell_e/d)^2}$$

c = 0.8 for sawn lumber

c = 0.85 for round timber poles and piles

c = 0.9 for structural glued laminated timber or structural composite lumber

Find the maximum load, P_{max}

$$F_c' = 1300(1.6\ \ 0.8\ \ 0.7261)$$

$$= 1208\ psi$$

$$P_{max} = F_c'\ A = 1208(32.38) = 39115\ \#$$