

ARCH 324 STRUCTURE II

FACULTY: Prof. Peter Von Bulow

GSI: Faezeh Choobkar

Winter 2025

Recitation



8. Concrete Beam Analysis

Analyze the given composite floor system. Using a transformed section, determine peak stress values in both concrete and steel.

DATASET: 1

-2-

-3-

simple span

section width, b

17 FT

14 IN

section height, h

25 IN

max. aggregate size

0.75 IN

bar size number

5

the number of bars

6

stirrup bar size number

3

concrete cover

1.5 IN

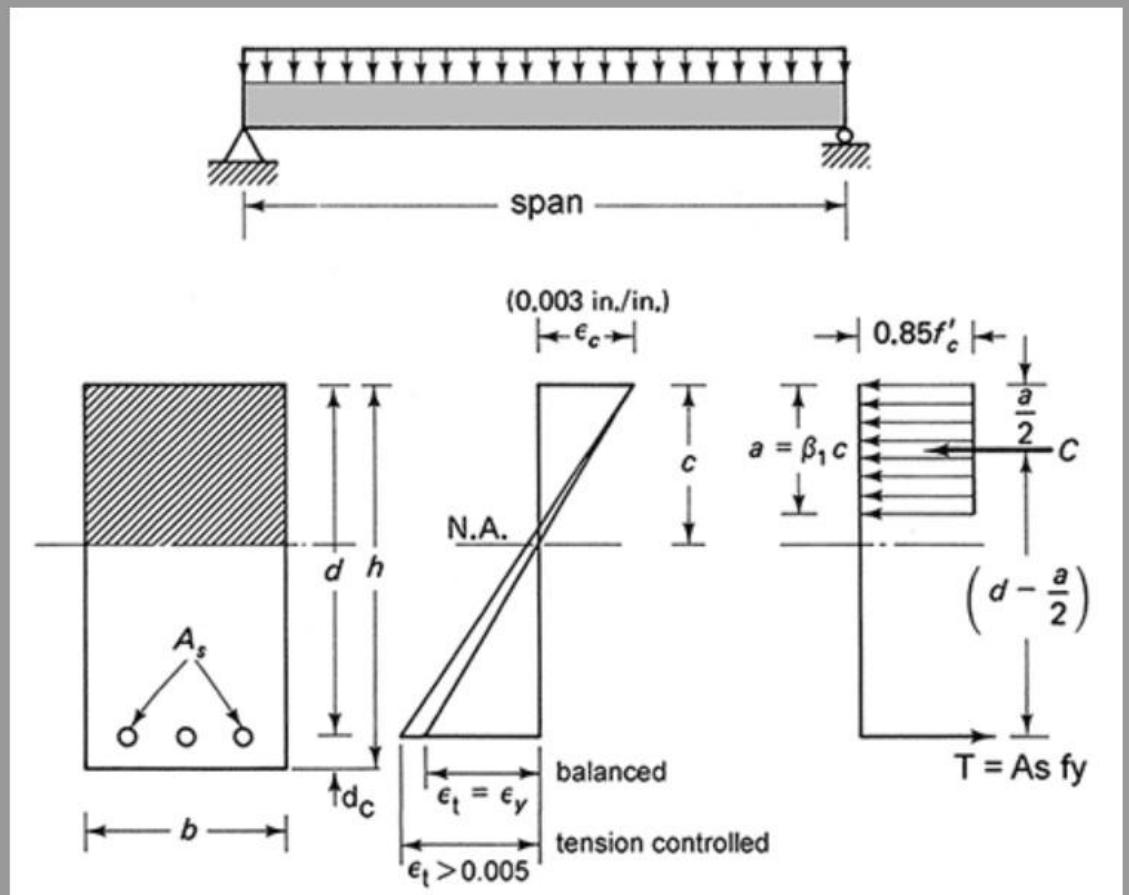
concrete ultimate strength, f'_c

6500 PSI

steel yield strength, f_y

60000 PSI

Logged in as: Prazen Choudhary

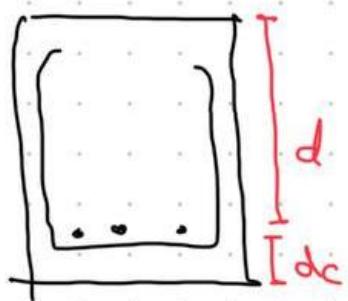


#	<u>Question</u>	<u>Your Response</u>	<u>Correct Answer</u>	<u>Score</u>
1	flexural steel bar diameter, db	IN	SUBMIT	
2	stirrup bar diameter	IN	SUBMIT	
3	distance from lower beam edge to center of flexural steel, dc	IN	SUBMIT	
4	distance from top beam edge to center of flexural steel, d	IN	SUBMIT	
5	minimum required area of steel, As,min (GREATER of the 2 criteria)	IN2	SUBMIT	
6	actual area of flexural steel, As	IN2	SUBMIT	
7	depth of concrete stress block, a	IN	SUBMIT	
8	factor beta_1		SUBMIT	
9	distance to Neutral Axis from top of beam, c	IN	SUBMIT	
10	strain in flexural steel, epsilon_t		SUBMIT	
11	strength reduction factor, phi		SUBMIT	
12	tensile force in the flexural steel, T	K	SUBMIT	
13	the nominal bending moment, Mn	K-IN	SUBMIT	
14	the factored bending resistance, phi Mn	K-IN	SUBMIT	
15	the factored design moment, Mu	K-FT	SUBMIT	

ASTM STANDARD REINFORCING BARS

Bar size, no.	Nominal diameter, in.	Nominal area, in. ²	Nominal weight, lb/ft
3	0.375	0.11	0.376
4	0.500	0.20	0.668
5	0.625	0.31	1.043
6	0.750	0.44	1.502
7	0.875	0.60	2.044
8	1.000	0.79	2.670
9	1.128	1.00	3.400
10	1.270	1.27	4.303
11	1.410	1.56	5.313
14	1.693	2.25	7.65
18	2.257	4.00	13.60

distance from lower beam edge to center of
flexural steel ,dc



$$= \text{Cover} + \text{Stirrup diameter} + \frac{\text{flexural bar diameter}}{2}$$
$$= 1.5 + 0.375 + \frac{0.625}{2} = 2.1875$$

distance from top beam edge to center of
flexural steel ,d

$$d = h - dc = 25 - 2.1875 = 22.8125$$

$$A_{s\min} = \begin{cases} 1. \left[\frac{3\sqrt{f'_c}}{f_y} b_w d = \frac{3\sqrt{6500}}{60000} (14)(22.8125) = 1.2874 \right] \\ 2. \left[\frac{200b_w d}{f_y} = \frac{200(14)(22.8125)}{60000} = 1.064 \right] \end{cases}$$

We should choose Max of 1 and 2 :

This is min requirement for the As

As : Actual area of flexure Steel

$A_s = \text{No. bars} \times \text{bar cross-sectional Area based on table}$

$A_s : 6 \times 0.37 = 1.86 < 1.2874 : A_{s\min}$

Calculate α : depth of Concrete stem block

$$\alpha = \frac{Asf_y}{0.85f'_c b} = \frac{(1.86)(60000)}{0.85(6500)(14)} = \frac{111600}{77350} = 1.442$$

$$\beta_1 = 0.85 - 0.05 \left(\frac{f'_c - 4000}{1000} \right)$$

$$= 0.85 - 0.05 \left(\frac{6500 - 4000}{1000} \right) = 0.725$$

C : Distance to N-Axis from top of beam

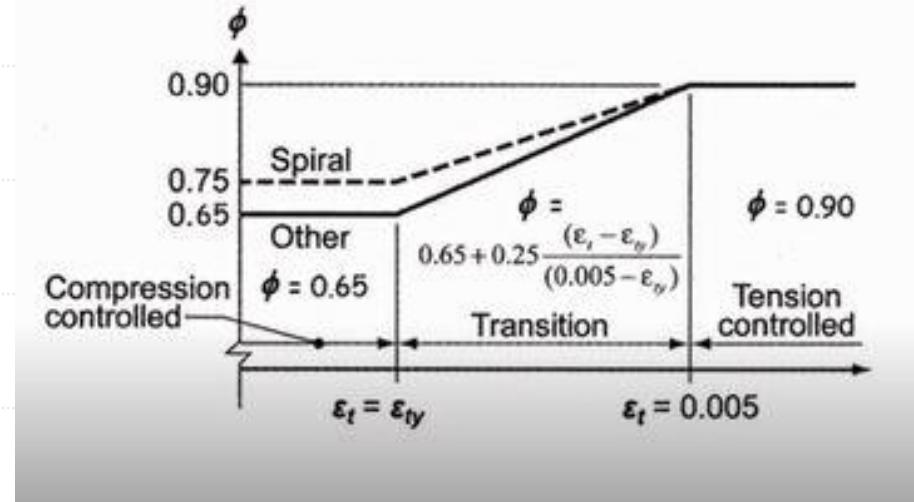
$$C = \frac{\alpha}{\beta_1} = \frac{1.442}{0.725} = 1.988$$

$$\epsilon_t = \frac{d-c}{c} (0.003) \geq 0.005$$

$$\frac{22.8125 - 1.988}{1.988} (0.003) = 0.031 \geq 0.005$$

Tension Controlled

$$\phi = 0.9$$



Calculate tensile force T

$$T = Asf_y = (1.86)(60000) = 111600$$

Calculate Nominal Bending Moment M_n

$$M_n = Asf_y \left(d - \frac{\alpha}{2}\right)$$

\equiv

T

$$M_n = (1.86)(60000) \left(22.8125 - \frac{1.442}{2}\right)$$
$$= 2465477.4 \xrightarrow{\div 1000} 2465.41 \text{ k-IN}$$

$$\phi M_n = 0.9 M_n = 2218.87$$

$$M_u = \phi M_n = 2218.87 \div 12 = 184.90 \text{ k-FT}$$

↓
max

- 
- Thanks for your attention 😊