

Architecture 324: Structures II

LRFD Steel Beam Analysis

Problem 4: Beam Capacity Analysis — Dataset 1

Given Data: W-Section: W12×35 F_y : 50 KSI E : 29,000 KSI
Span A: 25 FT (beam span) **Span B:** 11 FT (tributary width) **Floor DL:** 18 PSF
Assumptions: Fully braced ($L_b < L_p$) Deflection Limit: $L/180$
LRFD Load Combo: $w_u = 1.2 w_{DL} + 1.6 w_{LL}$

Part 1: Section Properties (AISC Table 1-1)

Q1 Plastic Section Modulus (Z_x)

Lookup: AISC Table 1-1 → W12×35

1. **Action:** Open AISC Table 1-1 (W-Shapes).

2. **Locate:** Row for W12×35.

Final Z_x : _____ IN³

Part 2: Flexural Capacity (Zone 1: $L_b < L_p$)

Q2 Nominal Bending Moment (M_n)

$$M_n = M_p = F_y \times Z_x$$

Final M_n : _____ K-IN

Q3 Factored Bending Resistance ($\phi_b M_n$)

$$\phi_b M_n = 0.90 \times M_n$$

1. **Calculate:** $0.90 \times$ _____ K-IN

Final $\phi_b M_n$: _____ K-IN

Q4 Factored Design Moment (M_u)

$$M_u = \phi_b M_n \quad (\text{set design at full capacity})$$

1. **Convert to K-FT:** $M_u = \phi_b M_n \div 12$
 $=$ _____ $\div 12$

Final M_u : _____ K-FT

Part 3: Load Capacity

From $M_u = w_u L^2 / 8$, solve for w_u . Then: $w_u = 1.2 w_{DL} + 1.6 w_{LL}$, solve for w_{LL} .

Q5 Total Factored Design Load (w_u)

$$w_u = \frac{8 M_u}{L^2}$$

1. **Numerator:** $8 \times M_u = 8 \times$ _____ $=$ _____

2. **Denominator:** $L^2 = (\text{Span A})^2 =$ (_____)² $=$ _____

3. **Divide:** _____ / _____

Final w_u : _____ KLF

Q6 Total Unfactored Dead Load (w_{DL})

$$w_{DL} = w_{\text{beam}} + w_{\text{floor}}$$

1. **Beam Self-Wt:** W12×35 ⇒ _____ PLF

2. **Floor DL:** _____ PSF \times Span B _____ FT $=$ _____ PLF

3. **Sum & Convert:** (_____) + (_____) \div 1000

Final w_{DL} : _____ KLF

Q7 Total Factored Dead Load ($w_{u,DL}$)

$$w_{u,DL} = 1.2 \times w_{DL}$$

Final $w_{u,DL}$: _____ KLF

Q8 Factored Live Load ($w_{u,LL}$)

$$w_{u,LL} = w_u - w_{u,DL}$$

1. **Subtract:** _____ - _____

Final $w_{u,LL}$: _____ KLF

Q9 Actual Beam Live Load Capacity (w_{LL})

$$w_{LL} = \frac{w_{u,LL}}{1.6}$$

1. **Unfactor:** _____ \div 1.6

Final w_{LL} : _____ KLF

Q10 Floor Live Load Capacity (LL)

$$LL = \frac{w_{LL} \times 1000}{\text{Span B}}$$

1. **Convert & Distribute:** _____ KLF $\times 1000 \div$ _____ FT

Final LL : _____ PSF

Part 4: Shear Check

Check shear zone via h/t_w , then verify $V_u < \phi_v V_n$.

Q11 Max. Factored Shear ($V_{u,max}$)

$$V_{u,max} = \frac{w_u \times L}{2}$$

1. **Calculate:** $w_u =$ _____ $\times L =$ _____ $\div 2$

Final $V_{u,max}$: _____ K

Q12 Web Area (A_w)

$$A_w = d \times t_w$$

1. **From Table 1-1:** $d =$ _____ IN $t_w =$ _____ IN

2. **Multiply:** _____ \times _____

Final A_w : _____ IN²

Q13 Factored Shear Resistance ($\phi_v V_n$)

$$\phi_v V_n = \phi_v \times 0.6 \times F_y (\text{steelyieldstress} = 50K) \times A_w$$

(for zone 1, most beams are in this category)

1. $V_n = 0.6 \times F_y \times A_w$: $0.6 \times$ _____ \times _____

2. **Factor:** $\phi_v V_n =$ _____ \times _____

Final $\phi_v V_n$: _____ K

Q14 Shear Adequacy Check

1. **Compare:** $V_{u,max}$ _____ K vs. $\phi_v V_n$ _____ K

2. $V_u < \phi_v V_n$? _____ (1 = Yes, 0 = No)

Part 5: Deflection Check

Unfactored total $D+L$ load; limit $L/180$.

Q15 Actual Deflection (Δ_{actual})

$$\Delta = \frac{5 w L^4}{384 E I_x} \quad (\text{all units: K, IN})$$

1. **Total Unfactored:** $w = w_{DL} + w_{LL}$

$=$ _____ $+$ _____ $=$ _____ KLF

Convert: $\div 12 =$ _____ K/IN

2. **Span:** $L =$ _____ FT $\times 12 =$ _____ IN

3. **From Table 1-1:** $I_x =$ _____ IN⁴

4. **Numerator:** $5 \times w \times L^4 =$ _____

5. **Denominator:** $384 \times E \times I_x =$ _____

6. **Divide:** Num / Den

Final Δ_{actual} : _____ IN

Q16 Deflection Limit ($L/180$)

$$\Delta_{\text{limit}} = \frac{L \text{ (in)}}{180}$$

1. **Calculate:** _____ IN $\div 180$

Final Δ_{limit} : _____ IN

Q17 Deflection Adequacy Check

1. **Compare:** Δ_{actual} _____ IN vs. Δ_{limit} _____ IN

2. $\Delta_{\text{actual}} < L/180$? _____ (1 = Yes, 0 = No)

Deflection OK? _____